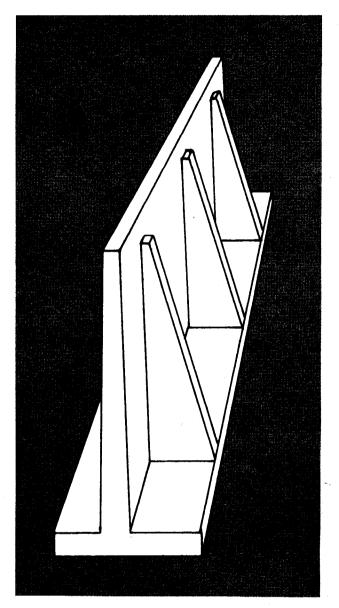
A WATER RESOURCES TECHNICAL PUBLICATION ENGINEERING MONOGRAPH NO. 27



Moments and Reactions for Rectangular Plates

UNITED STATES DEPARTMENT
OF THE INTERIOR
BUREAU OF RECLAMATION

Engineering Monograph No. 27

Moments and Reactions for Rectangular Plates

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Division of Design

Denver, Colorado



United States Department of the Interior



BUREAU OF RECLAMATION

As the Nation's principal conservation agency, the Department of the Interior has responsibility for most of our nationally owned public lands and natural resources. This includes fostering the wisest use of our land and water resources, protecting our fish and wildlife, preserving the environmental and cultural values of our national parks and historical places, and providing for the enjoyment of life through outdoor recreation. The Department assesses our energy and mineral resources and works to assure that their development is in the best interests of all our people. The Department also has a major responsibility for American Indian reservation communities and for people who live in Island Territories under U.S. Administration.

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Preface

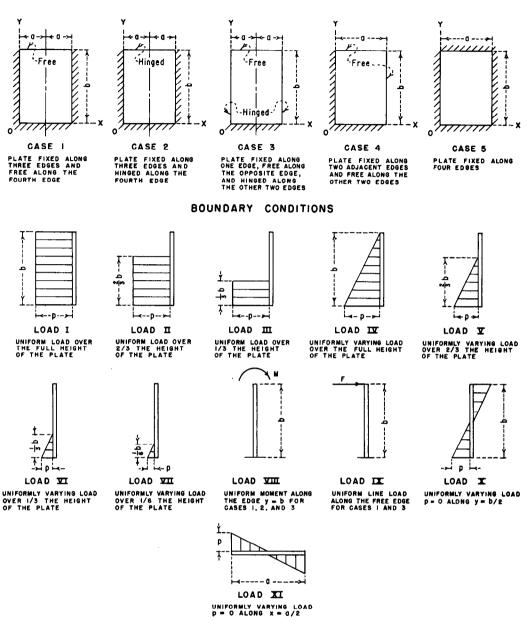
This monograph presents a series of tables containing computed data for use in the design of components of structures which can be idealized as rectangular plates or slabs. Typical examples are wall and footing panels of counterfort retaining walls. The tables provide the designer with a rapid and economical means of analyzing the structures at representative points. The data presented, as indicated in the accompanying figure on the frontispiece, were computed for five sets of boundary conditions, nine ratios of lateral dimensions, and eleven loadings typical of those encountered in design.

As supplementary guides to the use and development of the data compiled in this monograph, two appendixes are included. The first appendix presents an example of application of the data to a typical structure. The second appendix explains the basic mathematical considerations and develops the application of the finite difference method to the solution of plate problems. A series of drawings in the appendixes presents basic relations which will aid in application of the method to other problems. Other drawings illustrate application of the method to one of the specific cases and lateral dimension ratios included in the monograph.

Acknowledgments

The writer was assisted in the numerical computations by W. S. Young, J. R. Brizzolara, and D. Misterek. H. J. Kahm assisted in the computations and in checking the results obtained.

The figures were prepared by H. E. Willmann. Solutions of the simultaneous equations were performed using an electronic calculator under the direction of F. E. Swain.



LOADING CONDITIONS

NOTES

The various cases are analyzed for the indicated ratios of a/b.

Cases 1, 2, and 3: 1/8, 1/4, 3/8, 1/2, 3/4, 1, and 3/2.

Case 4: 1/8, 1/4, 3/8, 1/2, 3/4, and 1.

Case 5: 3/8, 1/2, 5/8, 3/4, 7/8, and 1.

All results are based on a Poisson's ratio of 0.2.

INDEX OF BOUNDARY AND LOADING CONDITIONS

—FRONTISPIECE

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6. My for Wall Slab at Supports

Introduction

CERTAIN COMPONENTS of many structures may be logically idealized as laterally loaded, rectangular plates or slabs having various conditions of edge support. This monograph presents tables of coefficients which can be used to determine moments and reactions in such structures for various loading conditions and for several ratios of lateral dimensions.

The finite difference method was used in the analysis of the structures and in the development of the tables. This method, described in Appendix

II of this monograph, makes possible the analysis of rectangular plates for any of the usual types of edge conditions, and in addition it can readily take into account virtually all types of loading. An inherent disadvantage of the method lies in the great amount of work required in solution of the large number of simultaneous equations to which it gives rise. However, such equations can be readily systematized and solved by an electronic calculator, thus largely offsetting this disadvantage.

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Method of Analysis

THE FINITE difference method is based on the usual approximate theory for the bending of thin plates subjected to lateral loads. The customary assumptions are made, therefore, with regard to homogeneity, isotropy, conformance with Hooke's law, and relative magnitudes of deflections, thickness, and lateral dimensions. (See Appendix II.)

Solution by finite differences provides a means of determining a set of deflections for discrete points of a plate subjected to given loading and edge conditions. The deflections are determined in such a manner that the deflection of any point, together with those of certain nearby points, satisfy finite difference relations which correspond to the differential expressions of the usual plate theory. These expressions relate coordinates and deflections to load and edge conditions.

In this study, for each load and ratio of lateral dimensions, deflections were determined at 30 or more grid points by solution of an equal number of simultaneous equations. A relatively closer spacing of points was used in some instances near fixed boundaries to attain the desired accuracy in this region of high curvature. For the a/b ratios 1/4 and 1/8, one and two additional sets, respectively, of five deflections were determined in the vicinity of the x axis. Owing to the limitations on computer capacity, these deflections were computed by solutions of supplementary sets of 20 equations whose right-hand members were functions of certain of the initially computed deflections as well as of the loads. In each case, the solution of the equations was made through the use of an electronic calculator.

Computations of moments and reactions were made using desk calculators and the appropriate finite difference relations. The finite difference relations used are discussed in Appendix II.

^{*}Numbers in superscript refer to publications in List of References on page 89.

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Results

FIGURES 1 through 36 present the results of these studies as tables of dimensionless coefficients for the rectangular components of bending moment and for reactions at the supports. The studies were carried out for the following edge, or boundary, conditions:

Case 1: Plate fixed along three edges and free along the fourth edge.

Case 2: Plate fixed along three edges and hinged along the fourth edge.

Case 3: Plate fixed along one edge, free along the opposite edge, and hinged along the other two edges.

Case 4: Plate fixed along two adjacent edges and free along the other two edges.

Case 5: Plate fixed along four edges.

The loads, selected because they are representative of conditions frequently encountered in structures, are:

Load I: Uniform load over the full height of the plate.

Load II: Uniform load over 2/3 the height of the plate.

Load III: Uniform load over 1/3 the height of the plate.

Load IV: Uniformly varying load over the full height of the plate.

Load V: Uniformly varying load over 2/3 the height of the plate.

Load VI: Uniformly varying load over 1/3 the height of the plate.

Load VII: Uniformly varying load over 1/6 the height of the plate.

Load VIII: Uniform moment along the edge y=b of the plate for Cases 1, 2, and 3.

Load IX: Uniform line load along the free edge of the plate for Cases 1 and 3.

Load X: Uniformly varying load, p=0 along y=b/2.

Load XI: Uniformly varying load, p=0 along x=a/2.

Plates with the following ratios of lateral dimensions, a, to height b, were studied for the first four cases: 1/8, 1/4, 3/8, 1/2, 3/4, 1, 3/2. The analysis was carried out for these cases using Loads I through IX and all dimension ratios, except that Load IX was omitted from Case 2 for obvious reasons, and Loads VIII and IX and the ratio a/b=3/2 were omitted from Case 4. It will be noted that for the first three cases, which have symmetry about a vertical axis, the dimension a denotes one-half of the plate width, and for the fourth, unsymmetrical case, a denotes the full width. For Case 5, lateral

dimension ratios of 3/8, 1/2, 5/8, 3/4, 7/8 and 1 were studied, subjected to Loads I, X, and XI. For this case, a and b denote the full lateral dimensions. All numerical results are based on a value of Poisson's ratio of 0.2.

The arrangement of the tables is such that each coefficient, both for reaction and moment, appears in the tables at a point which corresponds geometrically to its location in the plate as shown in each accompanying sketch.

Effect of Poisson's Ratio

A question which frequently arises is: What effect does Poisson's ratio have on the bending moments in a plate? For the plate fixed along four sides, a clear understanding of this effect

can be determined easily, since the deflections computed from finite difference theory are independent of Poisson's ratio. Futhermore, the bending moments at, and normal to, the fixed edges are unaffected by this factor. It is reasonable then to conclude that insofar as the moments which are most important in design are concerned, the maximum effect for this case will occur at the center of the slab.

Table 1 shows a comparison of maximum bending moment coefficients at the center of a uniformly loaded plate for several values of μ and for each ratio of a/b for which Case 5 was computed. For a change in Poisson's ratio from 0.2 to 0.3 it is noted that the maximum effect on the bending moment coefficient occurs at a/b=1, where the change in the coefficient is less than 8 percent.

Table 1.—Effect of Poisson's Ratio (μ) on Coefficients of Maximum Bending Moment at the Center of a Uniformly Loaded Rectangular Plate Fixed Along Four Edges

Values of M ₇ /pa ²												
a/b #	0	0.1	0.2	0.3								
0. 375	-0.0423	-0.0424	-0. 0424	-0. 0425								
0. 5 0. 625	-0.0403 -0.0358	-0.0407 -0.0367	-0.0411 -0.0376	-0.0415 -0.0384								
0. 75 0. 875	$ \begin{array}{c c} -0.0298 \\ -0.0235 \end{array} $	$ \begin{array}{r rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	$ \begin{array}{r rrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrrr$	-0.0337 -0.0283								
1. 0	-0.0177	-0. 0195	-0.0213	-0.0230								

	T		Γ	· · · · · · · · · · · · · · · · · · ·	N	M _x			<u> </u>			A	-		
1	y/b	R _v ×/o	0	0.2	T	, 		Τ	M _y						
	1.0	+.1249	<u> </u>	0.2	+.0002	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0	
1_	0.8	+.1248	+.0051	+.0024	+	0014	0024			+ 0005	+.0000	0	0	0	
	0.6	+.1247	+ .0052	+	+	0014	0023	0027		+	+.0000	0003	0005	0005	
1 11	0.4	+ 1250	+.0051	+ .0023		0014	0023	0027	+.0010	 	+ .0000	0003	0005	+	
م	0.2	+.1185	+	+ .0021	 	0013	0021	0024	+.0010	+.0004	+	0004	0006	+	
9	0	+.0504	0	+.0001	+.0003	+.0005	+.0006	+.0007	0		+.0016	+.0025		+	
		R _X R _y	+.0504	+.0116	+.0568	+ .0893	+.1084	+.1141		<u> </u>				1	
	1.0	+,2483	+.0209	+.0096	+.0007	0057	0096	0109	0	0	0	0	0	0	
<u>*</u>	0.8	+ .2523	+.0206	+ .0093	+.0006	0056	0093	0105	+.0041	+ .0019	+ .0002	0009	0016	0019	
1	0.6	+ .2513	+ .0205	+ .0093	+ .0006	0056	0093	0105	+ .0041		+ .0000	0013	0021	0023	
"_	0.4	+ .2512	+.0196	+ .0085	+.0003	0054	0088	0099	 	+ .0016	0004	0018	0027	0030	
% %	0.2	+ .1905	†	+.0053	0003	0039	0059	0065	+.0027		0011	0024	0032	0034	
"	<u> </u>	+.0295	0	+.0005		+.0020	 	+ .0027	0	+ .0023	+.0063	+.0101	+.0126	+.0135	
	1.0	+.3711	+.0295	+.0236	+.1131	+.1786	+.2174		<u> </u>	1 ^			T .		
ا ـــ ا	0.8		+.0466	+.0208	+.0012	0126	0218	0247 0235	+.0093	+.0042	+.0004	0 0022	0 - 0039	0	
3/8	0.6	+.3757	+.0442	+.0193	+.0007	0122	0198	0223			 		0038		
1	0.4	+.3541	+.0379	+.0155	0003	0107	0167	0186	+.0088	+.0036	0007	0039 0054	0059 0075	0065 0082	
ا م	0.2	+.2133	+.0210	+.0075	0009	0059	0085	0093	+.0042	+.0009	0017	0034	0044	0047	
q/p	0	0015	0	+.0010	+.0027	+.0043	+.0054	+.0058	0	+.0050	+.0135	+.0215	+.0269	_	
		R _X Ry	0015	+.0303	+.1666	+.2644	+ .3220	+.3410		1.0000	.0133	1.0270	1.0203	1.0200	
	1.0	+.5101				0233	0383	0432	0	0	0	0	0	0	
. ~	0.8	+.5331	+.0807	+.0349	+.0013	0218	0353	0397	+.0161	+.0068	0001	0049	0077	- 0086	
1/2	0.6	+ .4805	+.0712	+.0298	0000	0199	0313	0350	+.0142	+.0051	0026	0084	0120	0132	
n i	0.4	+.4148	+ .0545	+.0209	0014	0156	0233	0258	+.0109	+.0026	0043	0094	0125	0135	
g/b	0.2	+.1928	+.0250	+.0087	0009	0063	0089	0096	+.0050	+.0015	0003	0008	0008	0007	
0	0	- 0294	0	+.0019	+.0050	+.0080	+.0100	+.0107	0	+.0094	+.0252	+.0399	+ .0499	+.0534	
		Rx Ry	0294	+.0482	+ .2263	+ .3559		+ .4572		,					
	1.0	+ 8592	+.1788	+.0716	0010	0471	0726	0807	0	0	0	0	0	0	
34	0.8		+.1552	+ .0607	0020	0414	0630	0698	+.0310	+.0112	0027	0119	0172	0190	
"	0.6	+.5989	+.1207 +.0786	+ .0460 + .0280	0033 0033	0336 0214	0498 0306	0549 0333	+.0241	+.0071	0067 0049	0166 0100	0225 0127	0245 0135	
ا م	0.2	+.1185	+ .0289	+ .0109	+ .0009	0034	0049	0053	+.0058	+.0060	+.0115	+ .0186		+ .0262	
%	0	0694	0				+ .0227	+.0242	0	+.0212				+ 1212	
1 -		R _x R _y	0694			+.5271		+.6725		.02.12		.03.17			
	1.0	+1.2115	+ .2613	+.0883	0105	0654	0927	1008	0	0	0	0	0	0	
	0.8	+.9558	+.2146	+.0727	0097	0551	0774	0840	+.0429	+.0134	0051	0464	0224	0243	
-	0.6	+.6250	+.1547	+.0525	0083	0411	0566	0611	+.0309	+.0090	0069	0169	0222	0238	
",	0.4	+.3984	+ .0916	+.0305	0043	0216	0290	0310	+.0183	+.0069	+.0029	+ .0030	+ .0045	+.0053	
9/p	0.2	+.0434	+ .0303	+.0127	+.0047	+.0033	+.0042	+.0048	+.0061	+.0149	+.0339	+.0542	+.0689	+ .0742	
	0	0939	0	+.0074	+.0199	+.0311	+.0384	+.0409	0	+.0369	+.0996	+.1556	+.1919	+ . 2043	
		R _x R _y	0939		+ .4453	+.6760		+ .8450							
	1.0	+1.6267		+.0700	0345	0730	0844	0865	0	0	0 0069	0 - 0143	0165	0169	
3/2				+ .0565										+.0049	
10	0.6		+.1778	+.0399	0195 0056	0385 0116	0422 0101	0422 0090	+.0356					+.0685	
ما	0.4			+.0239			+.0273	+.0296			+		+.1699		
0	0.2	1168	0				+.0668	+.0702	0				+.3340		
1 1		Ry	1168					+1.0123							
						1		•							
		>-										i			
471												1	, 7		
	+	i }	Ĺ									/	p/p		
Free			!						,		M*/	≯ /-			
	1	Ł	į			Mome	nt = (Co	efficient) (pb²)		一て沈	R _x			

FIGURE 1.—Plate fixed along three edges, moment and reaction coefficients, Load I, uniform load.

Reaction = (Coefficient)(pb)

			<u> </u>						<u>I</u>					
	V/.	R. X/o	ļ <u>.</u>		T	M _X			<u> </u>	·	, ,	И _у		
	У/Ь			0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
_	0.8	+.0005	+.0000	+.0000	+.0000		0000	0000	0	0	0	0	0	0
×	0.6	+.0965	 	+.0017	+.0001	0001	0002			+.0001	-	+.0001	+.0001	+.0001
1 0	0.4		+.0051		+.0001	0014	0023	0026	-	+.0003		0003	0004	0005
%	0.2	+.1185	+.0048	+.0021	+.0001	0013	0021	0024		+.0004		0003	0006	0006 0007
9	0	+ .0503	0	+.0001	+.0003	+.0005	+.0006	+.0007	0		+.0016	+.0025	+.0031	+.0033
		R _K Ry	+.0503	+.0116	÷.0568	+.0893	+.1082	+.1149						
	1.0			+.0006		0002	0005	0007	0	0	0	0	0	0
74	0.8			+ .0022		0009	0019	0022	+.0008	+.0007	+.0007	+.0007	+.0007	+.0008
	0.6	+	+.0142	+.0060		0040	0064	0071	+.0028		0005	0016	0022	0025
- 4	0.4	+		+.0077 +.0051	0000	0052	0082	0092	+.0037	+.0013		0024	0034	0037
a/p	0.2	+.0299	0	+ .0005	+.0013	+.0039	+.0059	+.0065	+.0027	+.0006		0026	0034	0037
	H		+.0299	+.0241		+.1792	+.2177	+ .2304	<u> </u>	+.0024	+.0063	+.0100	+.0126	+.0134
	1.0		+.0038	+.0035	+.0012	0014	0032	0039	0	0	0	0	0	ō
3/8	0.8	+.0694	+.0127	+.0068	+.0012	0033	0061	0071	+ 0025	+.0018	+.0014	+.0011	+.0009	+.0008
, E	0.6	+.2724	+.0282	+.0113	0004	0081	0124	0138	+.0056	+.0015	0020	0047	0064	0070
11	0.4	+.3440	+.0330	+.0123	0013	0097	0142	0156	+.0066	+.0013	0036	0073	0096	0104
9/p	0.2	+.2181	+.0201	+.0067	0013	0058	0080	0087	+.0040	+ .0005	0025	0046	0058	0062
	0	+.0016	0	+.0010	+.0026	+.0041	+.0051	+.0055	0	+.0050	+.0131	+.0206	+.0256	+.0274
-		_	+.0016	+.0367	+.1705	+.2632	+.3165	+.3338						
1 1	1.0	-			+.0025	0043	0089	0106	0	0	0	0	0	0
2/	0.8		+.0257	+.0128	+.0015	0070	0122	0139	+.0051		+.0013	0000	0009	0012
1 ,, 1	0.4	+ .3937		+ .0138	0017	0123 0130	0181 0178	0199 0193	+.0084		0049	0097	~.0126	0136
	0.2			+.0064	0021	0062	0078	0083	+.0086	+.0001	0075 0027	0132	0167 0052	0178
9/p	0	0203		+.0018			+.0087	+.0092	0		+.0230			0054 +.0461
		R _X R _Y			+.2337	+.3439		+.4241		.0030	1.0230	1.0334	0434	7.0461
	1.0	+.0479	F.0445	+ .0243	+ .0028	0134	0230	0261	0	0	0	0	0	0
34	0.8	+ .2273	.0541	+ .0232	+.0002	0150	0236	0263	+.0108	+.0046	0006	0046	0072	- 0080
1 1	0.6	+.3991			0051	0185	0251	0271	+.0123	0007	0116	0194	0240	0255
"	0.4	+.4133					0196	- 0206	+.0106	0030	0137	0206	0243	0254
1%	0.2				0024	0044	0047	0046	+.0047	+	+.0008		+.0046	+.0054
"	0	0441		+ .0038				+.0167	0	1019. +	+.0452	+.0663	+.0792	+ .0835
 	1.0	R _X Ry +.1608	0441 +.0753		+.3413	+.4628 0209	0314	+ .5438				<u> </u>		
1 h	0.8				0032	0205	0290	0346 0315	0 +.0151	+ .0051	0 0007	0 0083	0	0
-	0.6	+ .4093		+.0183	0083	0211	0267	0282			0027 0160	0083 0241		0125
"	0.4	+.3959		+.0094	0086	0153	0175	0180	+.0110	-	0155	0201		0221
9	0.2	+.1392	+.0222	+ .0036	0011	0010	0001	+.0003						+.0237
[0	0523	0	+.0061	+.0136	+.0192	+.0226	+.0237	0	+.0307	+.0680	+.0962	+.1129	+ 1184
		RX					+.5977	+.6149						
_,	1.0	+.3060				0247	0290	0298	0	0	0	0	0	0
3/2	0.8	+.3324							+.0184			0099	0116 -	0120
1 " +	0.6	+.3934	-		0131	0194	0202		+.0152		0184	0221		0226
اما	0.4	+.3615 -		+ .0028 +	+ .0030	0104 +.0064	0093 + 0090	0087	+.0102		0107	0073		0018
9%	0.2	0489					+.0090 +.0331	+.0099						1719
1 h		Ry			+.5306			+.6737	<u> </u>	10000		+ . 1460	+.1657	1719
·			1						· · · · · · · · · · · · · · · · · · ·	···				
 	- ٥ مل											Y		
<i>y</i> .												↑ ~—		
-	<u> </u>	<u> </u>	7	1	η.							X	1/0	
Free		E									Mx	↓ —		
1			1-			Mom	ent = (Co	oefficient)(p b 2)		(1)	R.		
			• [1	React	ion = (C	oefficient	t)(pb)			Ry		
	ļ	E	Q.	· 🗐								0 /	V^-	 ^
	1		-								<i></i>	4	My	

Figure 2.—Plate fixed along three edges, moment and reaction coefficients, Load II, 2/3 uniform load.

					M	×					N	1 _y		
	У/Ь	R _X X/g		0.2	0,4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	1.0	0000	0000	0000	+.0000	+.0000	+.0000		0	0	0	0	0	0
🕏	0.8	0001	+.0000 +.0001	+.0000	+.0000	+.0000	0000	0000	+.0000 +.0000	+.0000	+.0000	+.0000	+.0000	+.0000
н	0.4	+.0286	+.0013	+.0006		0003	0006	0007	+.0003		+.0001	0000	0000	+.0001
9/p	0.2	+.1109	+.0044	+.0019	+.0000	0012	0019	0022	+.0009	+.0003	- 0001	0005	0007	0008
0,	٥	+.0505	0	+.0001	+.0003	+.0005	+.0006	+.0007	0	+.0006	+.0016	+.0025	+.0031	+.0033
	1.0	0013	+.0505	+.0125 0000	+.0570	+.0893	+.1083	+.0000						
_	0.8	0009	+.0001	+.0001	+.0001	0000	+.0000 0001	0001	+.0000	+ .0000	+.0001	+.0001	+.0002	+.0002
7,	0.6	+.0039	+.0011		+.0002	0003	0006		+.0002	+.0002	+.0003	+.0004	+.0004	+.0004
H	0.4	+.0653		+.0025	_	0015	0025		+.0011	+.0005	+.0000	0004	0006	0007
9/p	0.2	+.1703	+.0105	+.0034	0008	0032	0044	0047	+.0021	+.0001	0017	0032	0040	0043
	-		0 +.0354	+.0004	+.0011	+.0017	+.0021	+.0023	0	+.0022	+.0057	+.0087	+.0107	+.0114
	1.0	0075	0001	+.0002		0000	0002	0003	0	0	0	0	0	0
3/8	0.8	+ .0020	+.0012	+.0008	+.0003	0003	0006	0008	+.0002	+.0003	+.0004	+.0005	+.0005	+.0006
	0.6	+.0167	+.0039	+.0022	+.0004	0010	0019	0022	+.0008	+.0007	+.0006	+.0006	+.0005	+.0005
"	0.4	+.0947	+.0103	+.0039	0004	0030	0044	~.0048	+.0021	+.0006	0008	0019	0027	0029
9/p	0.2	+.1940	+.0140	+.0031	0025 +.0020	0043 +.0029	+.0049	0051 +.0037	+.0028	0009 +.0041	+.0042	+.0061 +.0146	0072	0076
		R _X Ry	+.0171	+.0623		+.2211	+.2485	+.2567		T.0041	T.0099	T.0146	+.0175	+.0184
	1.0	0135	+.0008	+.0011	+.0004	0004	0010	0012	0	0	0	0	0	0
~	0.8	+.0103	+.0032	+.0019	+.0005	0008	0017	0020	+.0006	+.0005	+.0005	+.0005	+.0005	+.0005
	0.6	+.0305	+.0068	+.0033	+.0003	0019	0031	0035	+.0014	+.0009	+.0004	1000	0004	0005
"_	0.4	+.1090	+.0125	+.0039	0011	0038 0040	0050 0043	0053 0043	+.0025	+.0002 0019	0021	0039	0050	0054
9%	0.2	+.1846	+.0131	+.0015	0027 +.0029	+.0040	+.0047	+.0049	+.0026	+.0066	0052 +.0144	+.0202	0081 +.0236	0083 +.0247
		R _X R _y	+.0093	+.1028	+.2064	+.2593	+.2836	+.2905		1.0000	1.0144	1.0202	1.0230	7.0247
	1.0	0086	+.0053	+.0036	+.0007	0017	0033	0038	0	0	0	0	0	0
3/4	0.8	+.0270	+.0079	+.0038	+.0003	0022	0036	0041	+.0016	+.0009	+.0004	0001	0005	0006
1 1	0.6	+.0439	+.0107	+.0040	0006	0031	0043	0047	+.0021	+.0009	0006	0019	0028	003
"_	0.4	+.1131	+.0140	+.0025 0006	0024 0030	0042 0031	0048 0029	0050 0027	+.0028	0011 0036	0044 0063	0066 0070	0078 0070	0082 0069
%	0.2	+.0046	0	+.0024	+.0045	+.0058		+.0067	0	+.0119	+.0225	+.0291	+.0326	+.0336
1 1		R _X Ry	+.0046	+.1652	+.2588	+.2967	+.3116	+.3156						
	1.0	+.0066	+.0100	+.0050	+.0002	0030	0047	0052	0	0	0	0	0	0
-	0.8	+.0374	+.0113	+.0046	0003	0031	0045	0049	+.0023	+.0011	+.0001	0008	0014	0015
"	0.6	+.0459	+.0126	+.0035	0013 0029	0035 0039	0044 0041	0046 0041	+.0025	+.0006 0022	0015 0057	0031 0074	0040 0081	0043 0082
٥/p	0.2	+.1696	+.0095	0016	0026	0031	0017	0016	+.0019	0046	0058	0051	0042	0039
6	0	+.0083	0		+.0058	+.0071	+.0078	+.0080	0		+.0288		+.0389	+.0399
		R _X Ry	+.0083	+.2073	+.2860	+.3134	+.3239	+.3267						
	1.0	+.0281	+.0146	+.0043	0014	0037	0043	0045	0	0	0	0	0	0
3/2	0.8	+.0453		+.0034	0017 0023				+.0028	+.0009	0005 0023	0013 0033	0016	0017
1 11	0.6	+.0409	+.0130	+.0017 0009	0029	0032 0028	0032 0025	0024	+.0028	0002	0060	0061	0035 0056	0036 0054
اما	0.2	+.1801	+.0067	0021	0015	0006	0001	+.0000	+.0013	0048	0030	0004	+.0014	+.0020
0	0	+.0221	0		+.0075	+.0088	+.0095	+.0097	0	+.0247	+.0375	+.0441		+.0483
		ŘŽ	+.0221	+.2561	+.3114	+.3277	+.3334	+.3349			-			
Free				:				efficient			Mx	Rx B	<u>/</u> 0	
mm	· · · · · · · · · · · · · · · · · · ·		-x 1	← P→	x	Nedt (1	- (O		.,(pu)		W	IVE SIGI	My N CONV	→ X

Figure 3.—Plate fixed along three edges, moment and reaction coefficients, Load III, 1/3 uniform load.

					N	A _x				*	N	l _y		
	y/b	R _X X/0	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	1.0		+.0004	+.0002	+	0001	0002	0002	0	0	0	0	0	0
8/	0.8	+.0251	+.0011	+.0005		0003	0005	0005	-		+.0000	0000	0001	
l u	0.6	+.0751	+.0021	+.0009	+.0001	0006	0009	0016	+.0004	1	+.0000	0001	0002	
	0.2	+.0942	+.0038	+.0016	+.0000	0010	0017	0019	+.0006	+.0003	0000	0002	0003	
9/p	0	+.0460	0		+.0003	+.0005	+.0006	+.0006	0	+.0005	+.0014	+.0023	+.0028	+.0030
		R _x Ry	+.0460	+.0136	+.0543	+.0839	+.1004	+.1056		1			1.0020	1.0030
	1.0	+.0147	+.0022	+.0012	+.0002	0006	0012	0014	0	0	0	0	0	0
1/4	0.8	+	+.0046		+.0002	0012	0021	0024	+.0009	+.0005	+.0002	0000	0002	0002
- 11	0.6	+.1015	+.0083		+.0002	0023	0038	0042	+.0017	+.0007	0000	0005	0009	0010
4	0.4	+.1494	+.0114	+.0049	+.0001 0004	0032 0030	0051	0057 0047	+.0023	+.0008	0004	0013	0019	0021
9/p	0.2	+.0304	0	+.0004	+.0010	+.0016	+.0020	+.0021	0	+.0020	+.0052	0022 +.0081	0029 +.0100	+.0107
			+.0304	+ 0309		+.1563	+.1856	+.1950	<u> </u>	17.0020	1.0032	1.0081	1 .0100	+.0107
	1.0	+.0189	+.0066	+.0040	+.0008	0020	0039	0045	0	Q	0	0	0	0
3/8	0.8	+.0885	+.0117	+.0056	+.0006	0031	0054	0062	+.0023	+.0012	+.0004	0002	0005	0007
	0.6	 	+.0176	+.0075	+.0001	0049	0079	0088	+.0035	+.0013	0006	0020	0029	0032
"	0.4	+.2107	+.0208	+.0079	0007	0061	0090	0099	+ 0042	+.0009	0019	0042	0056	0061
0/p	Q .2	 	+.0145	+.0045	0012	0042	0057	0061	+.0029	+.0001	0022	0039	0048	0051
	0	+.0102 R _x Ry	+.0102	+.0008	+.0020	+.0030	+.0038	+.0040	0	+.0039	+.0099	+.0152	+.0188	+.0200
-	1.0	+.0326	+.0102	+.0474	+.1488	+.2154	+.2526	+.2645 0097	0	0	0	0		
۱ ۵	0.8	+.1315		<u> </u>	+.0007	0059	0099	0112	+.0043	+.0020	+.0002	0011	0 0019	0 0022
1/2	0.6	+.1972	+.0273	+.0108	0005	0079	0119	0132	+.0055	+.0015	0020	0047	0064	0070
1 11	0.4	+.2421	+.0277	+.0092	0019	0082	0115	0125	+.0055	+.0004	0042	0076	0097	0104
q/p	0.2	+.1607	+.0160	+.0041	0017	0044	0055	0058	+.0032	0002	0026	0039	0044	0046
0	0	0045	0		+.0033	+.0050	+.0061	+.0065	0	+.0068	+.0167	+.0252	+.0307	+.0325
		R _X R _Y	0045		+.1942	+.2699	+.3108	+.3236	<u>.</u> .					
	0.8	+.1061	+.0406	+.0196	+.0013	0115	0190	0214	0	0	0	0	0	0
3,4	0.6		+.0426	+.0145	0003 0026	0119 0124	0184 0174	0205 0189	+ .0087	+.0031	0012 0055	0042 0102	0061	0067
1 11	0.4		+.0349	+ .0091	0039	0102	0130	0138	+.0070	0011	0075	0102	0130 0137	0139 0143
9/p	0.2	+.1337	+.0163	+.0031	0017		0033	0033	+.0033	+ .0001				+ .0035
0	0	0196	0	+ .0028	+.0064	+.0093	+ .0111	+.0117	0	+.0139		+.0465		+.0584
		R _X R _Y	0196		+ . 2666	+.3496	+.3923	+.4055						
1 1	1,0		+.0644	+.0253	0013	0172	0252	0276	0	0	0	0	0	0
-	0.8		+.0601	+.0210	0028	0161	0226	0245	+.0120	+.0034	0026	0065		0095
"	0.6	+.2485		+.0149	0047 0049		0189 0118	0201 0122	+ .0103	+.0003		0125	0151	0159
9%	0.2	+.1108			0006	0006		+.0003	+.0074	+.0021		0099 +.0116		0107 +.0175
	0	0241		+.0044			+.0161	+.0169	0					+ .0845
		R _X R _y	0241	-		+.4038		+.4584						
	1.0	+.3127	+.0857	+.0207	0087	0199	0232	0238	0	0	0	0	0	0
1 %	0.8	_			0086	0172	0194	0198	+.0146	+.0023	0042	0072	0082	- 0085
1 " 1	0.6			+.0094		0134	0142	0141	+.0112	0013	0077	0096	0096	0094
ا مي ا	0.4		+.0359			0065	0057	0053	+.0072	0021				+.0059
6	0.2	0204		+ .0021 -				+.0076 +.0252						+ .0474
1 h		Ry		+ .2452				+ .5047	0	+.0396	+.0791	+.1062	+ .1214	+.1262
/ /		1					nt = (Go on = (Co				Mx (R _x	\sqrt{0}	→ x
			<u>_</u> v								w	Y	М,	

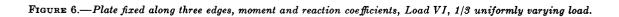
FIGURE 4.—Plate fixed along three edges, moment and reaction coefficients, Load IV, uniformly varying load.

						Λ _x			T		N			
1	y/b	R _X X/a	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.0	T
-	1.0	_	+.0000				0000			0.2	0.4	0.6	0.8	1.0
1/8	0.8	+.0002	+.0000	+.0000		+	0000			+.0000				+.0000
_	0.6	+.0142	+ .0006		+.0000	0002	0003	0003		+.0001		0000	0000	0000
10	0.4	+.0499	+.0021	+.0009		0006	0009		+.0004	+.0002	+.0000	0001	0002	0002
\\ \phi \phi	0.2	+.0818	+.0032		+ .0000	0009				+.0003	0001	0003	0004	0005
"	<u> </u>	+.0437	0	+.0001		+.0004			0	+.0005	+.0014	+.0021	+.0026	+.0028
	1.0		+.0437	+.0145	+.0537		+.0959		<u> </u>			,		
1 _	0.8	+	+.0006	+.0004		0000	0001	0001	0	0	0	0	0	0
1/4	0.6	+.0323			+	0008	0014	0017		+.0001		+.0002	0003	+.0003 0000
111	0.4	+.1011	+.0075	+.0031	0000	0021	0033	-		+.0005	0003	0010	0014	0015
م ا	0.2	+.1286	+.0084	+.0030	0005	+		0038	+.0017	+.0002	0010	0020	0027	0029
0	0	+.0308	0	+.0004	+.0009	+.0014	+.0018	+.0019	0	+.0019	+.0047		+.0088	+.0093
ļ		R _X Ry	+.0308		+.1011		+.1695	+.1773						
	1.0	0104	+.0005	+.0007	+.0003	0002	0006	0008	0	0	0	0	0	0
3/8	0.8		+.0027	+.0017	+.0004	0007	0014	0016	+.0005	+.0005	+.0005	+.0006	+.0006	+.0006
1 11	0.6	+.0558	+.0074	+.0034	+.0002	0020	0034	0038	+.0015	+.0008	+.0001	0004	0007	8000.
ا ما	0.4		+ 0131	+.0047	0006	0039	0056	0061	+.0026	+.0004	0015	0031	0041	0044
q/ _p	0.2	+.1457 +.0155	0	+.0031	+.0012	+.0024	+.0030	+.0032	+.0023	0002	0023	0038	0047	0050
	_	R _x R _y	+.0155	+.0549	+.1389	+.1907	+.2183	+.2269		+.0033	+.0081	+.0122	+.0149	+.0158
\vdash	1.0	-	+.0025		+.0007	0009	0021	0025	0	0	0	0	0	-
,	0.8		+.0063	+ .0034	+.0006	0017	0031	0036	+.0013				+.0003	
1/2	0.6	+.0763	+.0118	+.0049	- ,0001	0034	0052	0058	+.0024	+.0009	0005	0016	0024	0026
- 11	0.4	+.1567	+.0165	+.0049	0016	0050	0067	0072	+.0033	0001	0033	0056	0071	0075
q/ _p	0.2	+.1409	+.0119	+.0023	0018	0034	0040	0041	+.0024	0008	0032	0045	0052	0054
0	0	+.0062	0	+.0011	+.0026	+.0037	+.0045	+.0047	0	+.0056	+.0129	+.0187	+.0223	+.0235
			+.0062			+.2283	+.2546	+.2625						
]]	1.0		+.0108		+.0010	0034	0060	0069	0	0	0	0	0	0
34	0.8			+.0065	+.0003	0040	0064	0072	+.0029		+.0002	0008	0014	0017
1 1	0.6	· · · · · · · · · · · · · · · · · · ·	+.0178		0013	0053	0072	0078	+.0036		0024	0046	0060	0065
ا ا	0.2		+.0190	+.0007	0031 0021	0058 0026	0068 0025	0071 0024	+.0038	0017 0016	0061 0031	0089	0104	0109
2	0.2	0003	0	+.0021				+.0072	0			0031 +.0300	- 0028 + 0345	0026
		R _x R _y	0003	+.1334				+.3005	_ ']	0101	.0220	* .0300	7.0343	1.0333
	1.0	_		+.0088	+.0001	0055	0084	0093	0	0	0	0	0	0
_	0.8	+.0718	+ .0202	+.0077	0007	005 5	0079	0086	+.0040	+.0016	0003	0019	0028	0031
,, [0.6	+.0990	+ .0208	+.0055	0023	0060	0075	0079	+.0042	0001	0038	0063	0076	0081
	0.4	$\overline{}$	+.0189	+.0021	0037	0055	0060	0061	+.0038	0031	0074	0094	0102	0104
9%	0.2		+.0098	0002	0017	0014	0010	8000	+.0020	0017	0014	+.0004	+ .0020	+ .0026
1 1		+.0011						+.0092	0	+.0158	+.0300	+ .0393	+.0445	+.0461
			+.0011	+.1712	\rightarrow			+.3202						
~	1.0 0. 8	+.0673				0066	0078 0067	0080	+.0050	0	0 0012 ·	0 0025	0 0030 -	0031
3,	0,6			+.0037	0030		0056							0064
11	0.4		+.0168	0004	0038		0035	0033			0069	0065		- 0051
_o	0.2		+.0075	0007				+ .0019			$\overline{}$		+.0118	
0 [0	+.0110		+.0051				+.0122				+.0536 -	+.0591	+.0608
		Ry	+.0110	+ 2226	+.2954	+.3224	+. 3329	+.3356						
	*											~		
← 0>	o -	بو										i		
F;			_		_							\mathcal{L}		
Free					[w /	41	P	
	!				1	Mom	ent = (Co	a e f f iainn	41/ab21		M*	<u>זיין −</u>	-1	
				¥								Rx	1	
		1		<u> </u>	1	React	ion = (C	oetticien	T)(pb)			Ry	47	→ X
	!			a E							بر	<u> </u>	ν	•
				1 <i>E</i>							🛩	Ψ	/ My	
<i></i>	Viiin.	,,,! !	- -x	Y	ш.						W	·		
				-∍ip	- 4-						POSITI	VE SIGN	ONVI	ENTION

FIGURE 5.—Plate fixed along three edges, moment and reaction coefficients, Load V, 2/3 uniformly varying load.

MOMENTS AND REACTIONS FOR RECTANGULAR PLATES

1.0							Иx					N	/ly		
1.0		y/b	R _X X/O	0	0.2	0.4	0.6	0.8	10	0	0.2	T		1 0 0	T . ^
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0.6 +.0107 +.0035 +.0004000600080008 +.0007 +.0000000600080009000 0.4 +.0216 +.003300030008000800070006 +.00070009001500160015000 0.2 +.0789 +.002300090007000400030002 +.00050022001900120008000 0 +.0352		1.0	+.0063	+.0038	+.0012				$\overline{}$	0 1	0	0	0	0	0
0.6 +.0107 +.0035 +.0004000600080008 +.0007 +.0000000600080009000 0.4 +.0216 +.003300030008000800070006 +.00070009001500160015000 0.2 +.0789 +.002300090007000400030002 +.00050022001900120008000 0 +.0352		0.8	+.0118	+.0037	+ .0009	0004									0004
0.4 +.0216 +.00330003000800070006 +.000700090015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600150016001500160015001600160015001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001600160016001	ທີ	0.6	+.0107	+.0035	+.0004	0006	0008								0009
0.2 +.0789 +.002300090007000400030002 +.00050022001900120008000	- 11	0.4	+.0216	+.0033	0003	0008	0008	0007	0006			0015	0016 -	0015 -	0014
Ry +.0352 +.1434 +.1609 +.1653 +.1668 +.1671	٩ [0.2	+.0789	₩.0023	0009	0007 -	0004	0003	0002	+.0005			0012 -	0008 -	0006
	٥ ً	0	+.0352	0	+.0019	+.0027	+.0031	+.0033	+.0033	0	+.0097	+.0137	1.0155	1.0164	.0166
F: 1			Ry	+.0352	+.1434	+.1609	+.1653	+.1668	+.1671						
Ry (COETTICIENT) (PD)	0 > Free	0-										Mx	R ₁ ,		→ x



F	I		1	.	N	/ _x			ľ	-		VI y		
	У/ь	R _X X/o	-	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	T 0 =	1
— —	1.0	0000	0000	0000	0000				0	0.2	0.4	0.6	0.8	1.0
	0.8	0000	0000	0000	+.0000	+ 0000	+	+	0000	0000	+.0000	+.0000	0	0
_%	0.6	0000	0000	+.0000			+		0000	+ 0000	+.0000	+.0000	+.0000	+.0000
- 11	0.4	0003	+.0000	+.0000	+.0000		+		+.0000	+.0000	+ 0000	+ 0000	+.0000	+.0000
مي ا	0.2	+.0115	+.0004	+.0002	+.0000	0001	0002			+.0000	+.0000	+.0000	+.0000	+.0000
8	0	+.0235	0	+.0001	+.0002	+.0002	+.0003	+.0003	0	+.0003	+.0008	+.0011	+.0014	+.0014
		Ri Ry	+.0235	+.0207	+.0430	+.0559	+.0624	+.0650	† · · · ·	•			1	
	1.0	0000	0000	0000	+.0000	+.0000	+.0000	+.0000	0	0	0	0	0	0
<u>'</u> *	0.8	0001	0000	+ .0000	+.0000	+ .0000	0000	0000	0000	+.0000	+.0000	+.0000	+.0000	+.0000
-	0.6	0001	+.0000	+ .0000	+.0000	0000		0000	+.0000	+.0000	+.0000	+.0000	+.0000	+ 0000
"	0.4	+.0009	+.0002	+.0001	+.0000	0000		0001	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
q/ _p	0.2	+ 0174	+.0009	+.0003	0001	0002	0003	0004	+.0002	+.0001	0001	0001	0002	0002
	<u> </u>	+.0214	0	+ .0002	+ .0003	+ .0004		+.0005	0	+.0008	+.0016	+.0021	+.0024	+.0025
	<u> </u>		+.0214	+.0406	+ .0623	+.0720		+.0771		,		,	,	
	1.0	0004	0000	+.0000	+.0000	+.0000	0000	0000	0	0	<u> </u>	0	0	0
3/8	0.8	0001	+.0000	+.0000	+.0000	0000	0000	0000	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
	0.6	+.0004	+.0002	+.0001	+.0000	0000	0001	0001	+.0000	+.0000	+ .0000	+ 0001	+.0001	+.0001
"	0.4	+.0028	+.0005	+.0002		0001	0002	0003	+.0001	+.0001	+ 0001	+.0000	0000	0000
8	0.2	+.0219	+.0013	+.0003	0002	0004	0004	0004	+.0003	+.0000	0002	0004	0005	0006
	-	+.0196	+.0196	+.0002	+.0003	+.0004	+.0005	+.0005	0	+ 0008	+.0016	+.0022	+.0026	+.0027
1	1.0	0008	+ .0000	+.0000	+.0000	0000	+.0789 0000	+.0797		_				
1	0.8	+.0003	+ .0001	+.0001	+ 0000	0000	0001	0001	0	0	0	0	0	0
2	0.6	+.0010	+.0003	+.0002	+ 0000	0001	0001	0002	+.0000	+.0000	+.0000	+.0000		+ .0000
11	0.4	+.0036	+.0007	+.0003	0000	0002	0003	0003	+.0001	+.0001	0000	+ .0000	+ .0000	+.0000
	0.2	+.0214	+.0012	+.0001	0003	- 0003	0003	0003	+.0002	0001	0004	0001 0006	0001 0007	0002 0007
%	0	+.0202	0	+ 0002	+.0004	+.0006	+.0006	+.0006	0	+.0011	+.0022	+.0028	+.0031	+ .0032
1 1		R, Ry	+.0202	$\overline{}$	+.0740	+.0792	+.0811	+.0816	<u> </u>	1	* .0022		+.0031	1.0032
	1.0	0008		+.0002	+.0000	0001	0001	0002	0	0	0	0	0	0
] 😽 [0.8	+.0010	+ .0003	+.0002	+.0000	0001	0002	0002	+.00011	+ .0000	+.0000	+.0000	0000	0000
3/4	0.6	+.0017	+.0005	+.0002	0000	0001	0002	0002	+.0001	+.0001	+.0000	0000	0001	0001
" [0.4	+.0039	+.0008	+.0002	0001	0002	0002	0002	+.0002	+.0000	0001	0002	0003	0003
9/p	0.2	+.0206	+ .0010	0001	0003	0003	0002	0002	+.0002	0003	0005	0006	0007	0007
0	0	+.0233	0			+.0007	+ .0007	+.0007	0	+.0019	+.0029	+.0034	+.0036	+.0037
\vdash		_		+.0690	+.0791	+.0817	+.0825	+.0827						
1 1	1.0	0002	+.0004	 +	+.0000	0001	0002	0002	0	0	0	0	0	0
-	0.8	+.0015	+ .0005		0000	0001	0002	0002	+.0001	+.0001	+.0000	0000	0000	000 I
1 "	0.6	+.0019	+	+.0002	0001	0002	0002	0002	+.0001	+.0001	0000	0001	0002	0002
اميا	0.4	+.0036		+ .0001	0001	0002	0002	0002	+.0001	0000	0002	0003	0003	0003
8	0.2	+.0206	+ .0008	0002		0002	0002	0001	+.0002	0004	0006	0006	0005	0005
	0	+.0270	0			+ .0008	+.0008	+.0008	0	+.0023	+.0033	+.0038	+.0039	+.0040
\vdash						+.0826		+.0831						
ا ہا	0.8	+.0007 +.0019		+ .0002	0001	0002	0002	0002	0	0	0	0	0	-0
1%					0001	0001	0002			+ .0000	0000	0000		1000.
"	0.6	+.0017	+ .0006			1000.	0001			+.0000	0001	1000		- 0001
اما	0.4	+.0028 +.0228	+.0005		0001 0002	0001 0001	0001		+.0001	0001 0005	0002	0002 0004		0002 0003
8	0.2	+ 0332				+.0008		+.0009		+.0030		+.0042	1	+.0043
j t														
ر ۵»	Ry + .0332 + .0786 + .0825 + .0831 + .0834 + .0834													
Free		Moment = (Coefficient)(pb ²) Reaction = (Coefficient)(pb)												

Figure 7.—Plate fixed along three edges, moment and reaction coefficients, Load VII, 1/6 uniformly varying load.

	T		ľ			1 _x						A _y		
	У/ь	Rx X/O	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	100	1.0
—	1.0	+6.1755				 	+.1058		<u> </u>	+1.0000			0.8	1.0
	0.8	-3.0424	- 1024		†	+.0299	_	1 	+	0047	+.0096		+1.0000	+1.0000
_%	0.6	0473	0072		+.0014	+	+.0037	+	+	0016	0021	+.0208	+.0280	+.0304
1 11	0.4	+.0069	0001	0002	0002	+.0000	+.0001	+.0002	0000	0001	0002	0003	0029 0004	0030 0004
9/p	0.2	+,0006		+.0000	0000	0000	0000	0000		+ 0000	0000	0000	0000	0000
9	0	+.0000	0	+.0000	+.0000	 	+.0000	+ .0000	0	+.0000	1	+ 0000	+.0000	+.0000
		RX RY	+.0000	0000	0000	+ .0001	+.0001	+ .0001			1	1		1 .0000
	1.0	+11.3102	+.8269	+.4129	+.1755	+.0432	0233	0435	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
4	0.8	-5.0373	2113	0432	+ .0373	+.0733	+.0877	+.0914	0423	+.0215	+.0849	+.1351	+.1666	+.1773
->	0.6	6075	0707	0357	0041	+ .0193	+.0333	+.0379	0141	0096	0051	0007	+.0026	+.0038
- 11_	0.4	0291	0132	0092	0031	+.0028	+.0069	+.0084	0026	0032	0045	0057	0066	0069
1%	9.0	+.0114	0013	0014	0008	+ .0000	+.0007	+.0010	0003	0007	0014	0021	0026	0028
0		+.0051	0	+.0000	0000	0001	0001	0001	0	+.0001	0000	0003	0005	0006
		R _X N _Y		+.0061		+.0058	+.0016	+.0010						
	1.0	+14.6908			+.1309	0194	0856	_	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
3/8	0.8	-5.6047	1840	_	+.0825	+.0850	+.0767	+.0727	0368	+.0867	+ 2039	+.2901	+.3411	+.3578
	0.6	- 1.4017	1521		+ .0106	+.0481	+.0665	_	0304	0070	+.0220	+.0490	+.0676	+.0743
"	0.4	3181	0623			+.0167	+.0299	+.0343	0125	0094	0067	0037	0013	0003
9/p	0.2	+.0070	0141	0107	0040	+.0026	+.0071	+.0086	0028	0044	0070	0093	- 0108	0113
"	\vdash	+.0423	0	0003	0014	0028	0039	0044	0	0016	0071	0141	0197	0218
-		_	+.0423	+.0795	$\overline{}$	0593	1206	1433						
	1.0	 			+.0934	0518	1103	1262		+1.0000		+1.0000	+1.0000	
1/2	0.8	-5.5284	0776			+.0749		+.0362	0155	+.1650	+.3201	+ . 4235		+.4985
1 1	0.6	· · · · · · · · · · · · · · · · · · ·	1823			+.0667		+.0757	0365	+.0150		+.1345		+.1835
"	0.4	6544 0091	1073 0300	0424 0187		+.0332		+.0515	0215	3800	+.0088	+.0259		+.0429
9%	0.2	+.0985	0	0014	0042 0056	+.0065 0102	+.0124 0136	+ .0142	0060	0088	0120	0139		0149
		L		+.1413	0567	2627		0149	<u> </u>	0072	0279	0511	0681	0743
	1.0	+19.3123			+.0554	0813	4024	4506 1528	41,0000	41,0000		0000 T		
🚙	0.8			 +		+.0345	1371 0193	0363			+.5006	+1.0000		+.6669
\%	0.6					+.0631		+.0250			+.2172	+.3097	+.3637	+.3811
0	0.4	9946		+		+.0449		+.0326		+.0219	+.0788	+.1296	$\overline{}$	+.1741
ا م ا	0.2	1100	0387			+.0117		+.0083	0077	0061	+.0020	+.0130	$\overline{}$	+.0249
%	0	+.1491	0	0054	0138	0195	0222	0229	0	0271	0689	0977	$\overline{}$	1145
		R _X R _y	+.1491	0424	4227	6344	7143	7315						
	1.0	+20.8157	+1.7779	+.2069	1598	2961	3543	3712	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
_ [0.8	-4.4625	+ .4653	+.3317	+.1241	0007	0631	0819	+.0931	+.4346	+.6176	+.7031	+.7408	+.7515
1 [0.6	-1.9908	+.0543	+.1755	+.1153	+.0420	0044	0196	+.0109	+.1828	+.3423	+.4443	+.4975	+.5138
	0.4	-1.1509	0490	+.0715	+ .0727	+.0431	+.0193	+.0110	0098	+.0728	+.1714	+.2487	+.2949	+.3101
9/p	0.2	2816	0306	+.0179	+.0262	+ .0215	+.0172	+.0158	0061	+.0155	+.0568	+.1011	+.1338	+.1457
	0	+. 1323	0	0096	0122	0064	+ .0008	+.0039	0	0480	0610	0321	+.0040	+.0193
\vdash					7216	6791	5624	5103						
	1.0	+22.7458			+.0139	0768	1026					+1.0000		
3%	0.8	-4.4642						0474			_		+. 8395	
"	0.6			+ 2360										+.7012
اما	0.4						+.0498				+.3610			+.5757
% 9/P	Ø. 2 0	-	0061		+.0523		+.0737	+.0787			+.2247			+.4846
1 1		+.0256 Ry	+.0256		+.0169 7706	+.0536 3660	+.0801 1029	+.0894 0180	0	0515	+.0846	+ . 2679	+.4003	¥.4403
, 			.0236	. 3002	. 1 100	. 3000	. 1029	.0180	-					
	مال											Y		
Free					Y M			oefficient oefficien:			-		1/0	 ×
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,			'_x								W	IVE SIGI	,	ENTION

FIGURE 8.—Plate fixed along three edges, moment and reaction coefficients, Load VIII, moment at free edge.

RESULTS 15

				<u></u>	N	1 _x			Ī		ı	vl _y		
	y/b	R _X X/O	+	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	0.8		+.0471	+	+ 0007	+		·	0	0	0	0	0	0
<u>8</u>	0.6	0073	+ 0000	4	+.0006	•		·		+.0007		+.0014	+ 0017	
11	0.4	+.0005	- 0000	4	+ 0000		• • • • • • • • • • • • • • • • • • • •		0000	•	∳ – - +	- 0000	*	+.0002
م ا	0.2	+.0000	- 0000	0000					0000	0000	+	0000	4	- 0000
9	0	+.0000	0	+.0000	0000	0000	0000	0000	0	+.0000	0000	- 0000	0000	
		R _X Ry	+.0000		+.0000	+.0000	+.0000	~.0000						
	1.0	+2.3750	•	+.0587		0395	+	0667	0	0	0	0	0	0
- 4	0.8	+.1072 0306	+ .0278	4		0058	•			+.0067			+.0131	
111	0.4	0120	•	į.	+ -	+	+ .0001	* -	0001	+	+ .0004	+	+.0043	
9/p	0.2	0020	0002		+.0000	<u></u>	+ .0001		0000	+			+.0001	
0	0	+.0002	0	0000	0000	0000	0000	0000	0	0000	0001	0001	0001	0001
<u> </u>			+ .0002	+.0005		0011	0013	0017		,				
	1.0	+3.3048	· ·	+.0947	4	0681	0996	1094	0	0	0	0	0	0
3/8	0.8	+.3317	+.0857	 -	+.0103	0190	0370	0431	+.0171	+	+.0226	+.0261		+.0291
11	0.6	0370 0433	+.0189 +.0020	+	+.0064	+.0003	0099 0015	0123	+.0038	+·	+.0107	~	+.0174	- 1
ما	0.2	0150	0004	+.0004		+.0004	+.0001	0000	+ 0004	+ 0017		+.0056	+.0070	- 1
q/p	0	0006	0	0000	0001	0001	0000	+	0	0002	0003	- 0003		0002
	Ť	R _x R _y	0006	0032	0087	0101	0095	0091	<u> </u>		.0000	.0000	.0002	0002
	1.0	+4.0661	+.3938	+.1268	0162	0957	1369	1497	0	0	0	0	0	-
2/1	0.8	+.6108		+.0846		0376	0672	0769	+.0331	+.0330	+.0366	+.0393	+.0407	+.0412
	0.6	+ .0095	+.0555	+.0391	+.0130	0109	0268	0324	+.0111			+ .0306	+.0349	
"	0.4	0659	+.0139	+.0140	+.0071	0015	0079	0103	+.0028	+ . 0063		+.0169	+.0205	
a/b	0.2	0470	+.0012	+.0036 0001	+.0028	+.0009	+.0008	+ .0014	+.0002	+.0022		+.0085	+.0107	
"		0139	0139	0341	0344	0141	+.0069	+ 0154	0	0004	+.0013	+.0043	+.0071	+.0082
-	1.0	+5.2885	+.6266	+.1803	0331	1463	2036	2213	0	0	0	0	0	-
3/4	0.8		+.3486	+.1514	+.0098	0788	1268	1420	+.0697	+.0618		+.0568	+.0551	+.0544
, w	0.6	+.1509	+.1613	+ 0957	+.0218	0346	0682	0791	+.0323	+.0421	+.0546	+.0633	+.0679	+.0694
11	0.4	1083	+.0588		+.0188	0088	- 0267	0327	+.0118	+.0240		+.0548		+.0666
9/p	0.2		+ .0096	+.0170	+.0116	+ 0050		0005	+.0019			+.0503	+	+.0672
"	0	- 0495 Rx Ry	0 0495	1823	+.0050	+.0815	+.2120	+.2598	0	+.0013	+.0250	+.0577	+.0839	+.0937
	1.0		+ .8094		\rightarrow	1818	- 2421	- 2601	0	.0	0	0	0	
	0.8		+.5022			1093							+.0655	+.0644
-	0.6	+.2160	+.2571	+.1333	+.0199	0529	0906	1021	+.0514	+.0697	+.0873	+.0978	+.1030	+ 1046
	0.4					0117		0372	+.0204	+.0500		+	+	+.1349
9%	0.2	1		+.0304					+ .0038	+		+		+.1764
	0	1014		+.0020				+.0523	0	+.0100	+.0843	+.1728	+.2379	+.2614
—		R ₁ R _y	1014 +.9388	3394		+.2910 1904	+.5246 2162	+.6042 2210	0	0 1	0	0	0	-
~	0.8	+7.3629 +1.8569						1423						+.1001
3/2	0.6				0015									
"	0.4			+.0862	+.0257		+.0058						+	+.3076
°	0.2	5823	+.0330	+.0427	+				+.0066					+.4378
0	0	- 1985		+.0125			+.1144		0	+.0623	+.2658	+.4538	+.5722	+.6117
		Ry	1985	3944	+.2295	+ . 7018	+.9426	+1.0130						
ſ	1											Y		
	0-			-								∤		
<i>F</i>)	 	- i ı	τ -	F								И	1/0	
Free											M.,	-	-/	
								pefficient			(t)	R.		
			.			React	ion = (G	oefficien [.]	t)(F)		γ	Ry	4	v
	!										ير	10 J		~ ^
											/	~	/ My	
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	m	,,,, , },\	Ĺ-x		π.						W			
	1										POSIT	VE SIG	N CONV	INTION

Figure 9.—Plate fixed along three edges, moment and reaction coefficients, Load IX, line load at free edge.

			Γ											
		T vi			·	1 _x					٨	A _y	,	
	У/Ь	Y ₀ →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
_	1.0	Rx Ry +.0063	+.0063	+.0809	+.0996	+.1127	+.1204	+.1229	0	0	0			
~	0.8	+.1242	+.0049		+.0001	0013	0022	0025	+.0010	+.0004	0001	0004	000 6	0007
l u	0.6	+.1254	+.0052	+.0023	+.0002	0014	0023	0027	+.0010	+.0005	+.0000	0003	0005	0006
1 %	0.4	+.1253	+.0051	+.0023	+.0002	0014	0023	0027	+.0010	+.0005	+.0000	0003	0005	0006
0	0.2	+.1185	+.0048	+.0021	+.0001	0013 +.0005	+.0006	+.0007	+.0010	+.0004	+.0001	+.0025	+.0006	0007
L		Ry ->	+.0504	+.0120	+.0565	+.0891	+.1082		├ ॅ	11.0000	1+.0010	1+.0025	14.0031	+.0033
		R _x R _y	0097		+.1053	+.1482	+.1728	+.1808						
<	0.8	0097 +.2366	+.0165	+.0067	0002	0047	0073	00 8 1	+.0033	+.0009	0	0	0	0
1 11	0.6	+.2557	+.0201	+.0088	+.0003	0056	- 0073	0102	+.0040	+.0009	0003	0026	0036	0039 0030
	0.4	+.2530	+.0196	+.0085	+.0002	0055		0099	+.0039	+.0015	0004	0019	0029	0032
1%	0.2	+.1908	+.0138	+.0053	0003	0039	0059	0066	+.0028	+.0007	0011	0024	0032	0035
1	-	+.0295	+.0295	+.0005	+.0013	+.0020		+.0027	•	+.0024	+ .0063	+.0101	+.0126	+.0135
 		R. R.	- 0466		+.1237	+.1983			<u> </u>					
_ &	1.0	0466	0	0	0	0	0	0	0	0	0	0	0	0
3,8	0.8	+.3265	+.0301	+.0109	0013	0089	0130	0143	+.0060		0038	0073	0095	0102
11	0.6	+.3819	+ .0403	+.0161	0007	- 0116	0177 0163	0197 0181	+.0081	+.0023	0028	0068	0094 0089	0103 0097
1%	0.2	+.2164	+.0211	+.0075	0010	0059	0085		+.0042	+.0008	0019	0038	0048	0052
"	0	0013	٥	+.0010	+.0027	+.0043			0	+.0051	+.0135		+.0269	+.0288
<u> </u>		Ry	0013	+.0310	+.1684	+.2663		+.3427						
1	1.0	0902	0902 0	0050 0	+.1570	+ .2529	+ 3034	+.3192	0	0	0	0	Ó	ō
\%	0.8	+.3904	+.0412	+.0128	0035	0126	0172	0186	+.0082	0005	~.0083	0143	0180	0192
1 11	0.6	+.4751	+.0572	+.0197	0036	0172	0242	0263	+.0114	+.0013	0083	0159	0207	0224
8	0.4	+.4302	+.0508		0033	0152	0212	0230	+.0102	+.0011	0072	0137	0177	0191
) °	0.2	+.2047	0	+.0078	0017 +.0050	0065 +.0078	0086 +.0096	0092 +.0102	+.0049	+.0009	0017 +.0248	0030 +.0388	0036 +.0480	0037 +.0512
L_		R _y		+.0571	+.2350	+.3590		+ . 4525		1.0054	1.0240	1.0000	1.0400	7.0312
		R _x R _y		+.0482		+.3343		+.3874						
***	1.0 C.8	1465 +.4416	+ .0490	0 + .0086	0 0090	0163	0 0189	0196	0 +.0096	00 6 0	0 0197	0 0296	0 0354	0
1	0.6	+.5465		+.0142	0116	0225	0265	~.0274	+.0139	0054	0233	0366	0447	0373 0473
"_	0.4	+.4698	+.0594	+.0121	0097	0185	0214	0221	+.0119	0039	0177	0277	0334	0353
8	0.2	+.1759		+.0053	0029	0051	0052		+.0051	+.0013		1900.+		+.0040
l	<u> </u>	0530 R _v →	0 0530	+.0041	+.0098	+.0144	+.0171	+.0180	0	+.0207	+.0492	+.0719	+.0854	+.0898
		R _x R _y		+.1304		+.3709		+.3969						
l _	1.0	1593	0	0	0	0	0	0	0	0	0	0	0	0
	0.8	+.4456	+.0465	+.0014	0129 0174	0164 0225	0166 0227		+.0093	0125 0137	0294 0363	0403 0512	0461 0592	0479
1	0.6	+.5491		+.0031	0142	0176	0172		+.0112	0101	0265	0367	0418	0617 0434
1%	0.2	+.1622		+.0018	0033	0027		0008					+.0095	+.0104
	0	0584	0	+.0067	+.0139	+.0187		+.0219	0	+.0334	+.0697	+.0934	+.1056	+.1093
<u> </u>	 	R _X R _Y		+.2379		+.5707		+.6218					_	
. ~	1.0	1387	0	0	0	0	0	0	0	0	0	0	0	0
m	0.8	+.4395		0086						0238				0547
1 11	0.6	+.5302	+.0531	0110 0090	0207 0160	0191 0139		0162	+.0106 +.0089	0289	0533 0376	0648 0445	0693 0468	0704 0474
%	0.2	+.1588	+.0184	0020	0018	+.0007			+.0037		+.0082		+.0147	
0,	0	0456	0	+.0115	+.0192	+.0225		+.0239	0					+.1194
!	<u></u>	R _y —	0456	+.3937	+.5702	+.6160	+.6244	+.6252						
Y		0										<u> </u>		
4	linged		T				ent = (C tion = (C				M.	R)M,	— → X
mm	mm	·////	X	÷ρ	*				POSITI	VE SIGN	CONVE	NTION		

FIGURE 10.—Plate fixed along three edges—Hinged along one edge, moment and reaction coefficients, Load I, uniform load.

	I		I			A _x			T		A	A _y		
	У/Ь	x/a →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	Ť	100	T
	1 , 0	R _x R _y	0005	0025	0007			+ 0020		0.2	0.4	0.6	0.8	1.0
~	1.0	0005	0	0	0	0	0	0	0	0	0	0	1 0	1 0
-	0.8	+.0076	+.0005	+.0003		0001					+.0001	+.0001	+.0001	+.0001
111	0.6	+.0967	+.0039	+.0017						+.0003			0004	
1%	0.4	+.1186	+.0048	+.0023	+.0001		0023		+.0010	+.0004	0000	0003	0005	0006
6	0	+.0504	0			<u></u>	+.0006		0		+.0016	+.0025	+.0031	+.0033
		Ry	+.0504				+.1083							
	<u> </u>	R _x R _y	0067 0	0196 0	0058 0		+.0128			T		T		
74	0.8	+.0259		+.0021		0009	- 0018	0 0021	0 +.0007	+.0006	+.0006	0	+.0007	+.0007
"	0.6	+.1889		+.0060					+.0028		0005		0023	0026
	0.4	+.2496		+.0077		+			+.0037	+.0013	0008	0024	0034	0037
1%	0.2	+.1917	+.0136		0004			0065		+.0006	0012	0026	0034	0037
1	 	+.0299 Ry	0 + 0299				+.0025		<u> </u>	+.0024	+.0063	+.0100	+.0126	+.0134
		R. Ry	0266	0511			+.0415							
_ co	1.0	0266	0	0	0	0	0	0	0	0	0	0	0	0
3/8	0.8	+.0626		+.0055			0051				+.0010			
ш	0.6	+.2728	+.0277	+.0109		0080			+.0055			0051	0068	
%	0.2	+.2185		+.0067	0013	0058					0025	0074		0062
	0	+.0016	0				+.0051		0	+.0050		+.0206		+.0274
		Ry →		+.0368			+.3167						<u> </u>	
		R _X R _y	0552	0738	0008	+.0494		+.0875						
2~	0.8	+.1009	+.0184	0 + 0081	+ 0001	0 0052	0 - 0082	- 0091	0 +.0037	+ 0019	0000	0 0018	0 0030	0 0034
1 1	0.6		+.0390			0117					0060			0155
. q/p	0.4		+.0425		0038		0173	0187	+.0085	0002		0140		0190
0	0.2		+.0226						+.0045		0030			0060
	0	0198 R _v →					+.0086		0	+.0091	+.0229	+.0351	+.0429	+.0456
		R. R.	0982				+.1247							
	1.0	0982	0	0	0	0	0	0	0	0	0	0	0	0
3,	0.8		+.0255		0030	0080			+.0051		0049		$\overline{}$	0135
10	0.6		+.0472		0079 0089	0150	0168		+.0094				0304 0303	0322
9/9	0.2	+.1871	+.0224		0035	0049	0047			0005		0022		- 0012
	0	0393	0	+.0038	+.0085	+.0121	+.0142	+.0148	0	+.0190	+.0427	+.0605	+.0708	+.0741
\vdash		Ry →					+.5029							
1 1	1.0	R _x R _y	1143 0	0 0 0	+.0802 0	+.1215 0	+.1357 0	+.1389	0 1	0 1	0 [0	0	0
-	0.8		+.0254		0057	0083	0086	0086		0021		0157		0202
0	0.6	+.3855				0153					0248			0419
\ 9	0.4	+.4165			0120	0141		0132						0377
0	0.2	+.1766					0018 +.0170				+.0585		+.0026	
	0	0412 Ry →					+.5347		0	+.0297	T.0000	T.0/62]	+.0850	+.0876
		R _X R _y	1143	+.0484	+.1189	+.1322	+.1322	+.1314			, .			
%	1.0	1143	0	0	0	0	0	0	0	0 7074	0 0 0	0	00241	0
"'	0.8		+.0361		0075	0071	0062		+.0042	0074		0219		0246 0477
"_	0.4		+.0340		0129	0112	0097		+.0068	0199		0384		0405
9/9	0.2	+.1778	+.0151	0028	0023	0004	+.0007	+.0010	+.0030	0011	+ .0018	+.0046	+.0061	+.0065
"	0	0262		+.0098		+.0179		+.0189	0	+.0489	+ .0779	+ .0896	+ .0935	+.0944
\Box		Ry →	0262	+.3699	+.5053 [+.5372	+.5425	+.5429						
Ţ												Y		
F	inged		T				ent = (C tion = (C				M:	R _x) M,	 ×
mm	nam	11177	^ -	⊷p⊹	1				POSITIV	E SIGN	CONVEN	ITION		

Figure 11. --Plate fixed along three edges---Hinged along one edge, moment and reaction coefficients, Load II, 2/3 uniform load.

					N	×					N	1 _y		
	y/b	½ / ₀ →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	1.0	R _x R _y	0000 0	~ .0000 0	0000 0	0000 0	0000 0	0000 0	0	1 0	0	0	0	Ö
>8	0.8	0001	+.0000			0000	0000				+.0000			+.0000
l n	0.6	+.0000		+.0001		0000	0000	0000		+.0000		+.0000		+ .0000
9/0	0.4	+.0286	+.0013	+.0006	+.0000	- 0003 - 0012	0006 0019	0007 0022	+.0003	+.0001	+.0001		0000 0007	0001
0,	0	+.0507	0	+.0001	+.0003	+.0005			0	+.0006	+.0016		+.0031	+.0033
		Ry —	+.0507	+.0124		+.0892								
1	1.0	Rx Ry 0002	0002 0	0012 0	000B	0003 0	+.0001	1.0002	0	T 0	0	0	0	T 0
74	0.8	0008	+.0001	+.0001	+.0001	0000	0001	0001	+.0000	+.0001	+.0001	+.0001	+.0002	+.0002
] 11	0.6	+.0040	+.0011		+.0002	0003	0006	0007	+.0002		+.0003			+.0005
9%	0.4	+.0652	+.0055			0015	0025 0044		+.0011	+.0005	+.0000	0004	0006	0007 0043
0	0.2	+.0354	0	+.0004			+.0021		0	+.0022	+.0057			
		Ry —	+.0354	+.0344		+.1704								
	1.0	R _x Ry 0027	0027 0	0071	0032 0	+.0005 0	+ .0030	+.0039	0	0	1 0	0	0	0
3/8	0.8	+.0019	+.0011	+.0007	+.0002	0002			+.0002					+.0005
i ii	0.6	+.0169		+.0021		0010				+.0007			+.0005	+ .0005
9%	0.4	+.0953	+.0100	+.0038		0029	0043		+.0020		0008 0037	0019 0058	0026 0070	0029
0	0	+.0186	0	+ .0008		+.0028		+.0036	0	+.0040	+.0095	+.0141	+.0170	
		Ry —	+.0186	+.0642		+.2221								
	1.0	Rx Ry0077	0077 0	01 3 6	0037 0	+.0043	+.0093	+.0110 0	0	0	0	0	0	0
~	0.8	+.0082	+.0026	+.0015	+.0003	0007	0012		+.0005	+ .0005		+.0004	+.0003	+ .0003
1 11	0.6	+.0302	+.0065		+.0002	0018	0029		+.0013	+.0008	+.0003	0002	0006	0007
9/0	0.4	+.1092	+.0124	+.0038	0012 0027	0038 0040		0053 0043	+.0025	+.0001	0022 0052	0040 0072	0051 0081	0055 0084
0	0.2	+.0094	0	+.0013		+.0040			0	+.0066		+ .0202	+ .0236	+.0246
		Ry —	+.0094		+.2066	+.2593	+.2835							
1	1.0	R _X Ry 0163	0163 0	01 <u>52</u>	+.0023	+.0130	+.0185	0201	0	0	0	ō	0	0
3,4	0.8	+.0159	+.0042		0002	0013	0017			+.0004	0001	0008	0012	0014
"	0.6	+.0416	+.0088	+.0027	0010	0027	0033	0035			0012	0027	0037	0040
ام. ا	0.4	+.1141	+.0132	+.0018	0027 0032	0041 0032	0044		+.0026	0014 0038	0049 0067	0073 0076	0086 0078	0091
%	0.2	+.0052	0	+.0024					0	+.0012	+.0222	+.0284	+ .0314	+ .0323
		Ry →	+.0052	+.1676	+.2599	+.2954	+.3085	+.3118				·		
	1.0	R _X Ry	0199 0	0088 0	+.0095	+ .0177	+.0206	+.0213	0	0	0	0	0	0
-	0.8	+.0176	+.0044	+.0010	0008	0014	0015	0015	+ .0009		0009	0018	0025	0027
- 11	0.6	+.0425	+ .0088		0019	0027	0027		+.0018	0003	0027	0046	0057	0060
۰ م	0.4	+.1110	+.0120	0003		0038 0024	0035 0020	0034 0019	+.0024	0030 0051	0070 0072	0092 0073	0102 0070	0105 0068
0	0.2	+.1749	+.0092	+ .0033	+ .0055	+.0065			0	+.0167	+ .0274	+ .0326	+ .0348	+ .0354
		Ry →	+.0099	+ 2129	+.2870	+.3085	+ .3147	+.3160						
	1.0	R _X Ry	0211 0	+ .0038	+.0172	+.0198	+ .0197	+ 0195	0	0	0	0	0	0
3/2	0.8			·	0012					0006				
] ",]	0.6	+.0385	+.0073	0010	0024	0022	0019	0017	+.0015	0019	0049	0065	0071	0072
	0.4	+.1044	+.0091	0026 0029		0030 0016	0026 0014	0025 0013	+.0018	0056 0065	0093 0070	0106 0065	~ .0110 0062	0061
9/0	0.2	+.0253	0	+.0047			+.0073		0	+.0237			+ .0367	
		Ry	+.0253								·			
Y	F)													
mm	POSITIVE SIGN CONVENTION												NTION	

Figure 12. --Plate fixed along three edges---Hinged along one edge, moment and reaction coefficients, Load III, 1/3 uniform load.

<u> </u>	1		<u> </u>		N	1 _x					N	A _y	·	
	y/b	X/0→	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
l _	1.0	0003	0003 0	+.0024	+.0064	+.0092	+.0109	+.0114	-	0	0	0	0	
~	0.8	+.0250			+.0000	0003		0005		+.0001	+.0000	0001	0001	0001
11	0.6	+.0500			+.0001	0006				+.0002			0002	0002
مي ا	0.4	+.0752			+,0001	0008		0016			+.0000		0003	
1%	0.2	+.0941	0 0		+.0000	0010		0019	8000.+1	+.0003	+.0014		+.0005	0006 +.0030
]	۳	Ry -	+.0459		+.0546		+.1003	+.1058	╁ ╵	14.0003	17.0014	1+.0023	17.0028	7.0030
		R _X R _y	0061	0102	+.0056	+.0169	+.0237	+.0260						
74	1.0	0061	0	0	0	0	0	0	0	0	0	0	0	0
	0.8				+.0001			0021		+.0004		0002	0004	0005
1 "_	0.4				+.0000		0051			+.0009			0019	0021
8	0.2		+.0102				0043			+.0004		0022	0029	
"	0	+.0304	0		+.0010				0	+.0020	+.0052	+.0081	+.0100	+.0107
L		Ry —	+.0304		+.1052				L					
İ	1.0	R _x R _y	0205 0	0283 0	+.0070	+.0313	+.0455	+.0502	0	1 0	0	0	0	-
3/8	0.8		+.0091	+.0039		0026	0041	0046		+.0007	0003	0010	0015	0016
1	0.6		+.0170			0048				+.0011			0035	0038
"	0.4	+.2120	+.0207	+.0078	0008	0061	0089	0098	+.0041	+.0009	0021	0044	0058	0063
%	0.2		+.0145	+.0045			0057	0061		+.0001	0023		0049	0052
1 1	<u> </u>	+.0102 R _v →	+.0102		+.0020			+.0040	-	+.0039	+.0099	+.0153	8810.+	+.0200
-		R _X R _y	0396		+.0153			+.0767						
1	1,0	0396	0	0	0	0	0	0	0	0	0	0	0	-
1/2	0.8	+.1042			0005		0060		+.0028	+.0006	0014	0030	0040	0044
1 11	0.6				0013		0104		+.0049		0031	0062	0082	0089
ا %	0.4	_			0023				+.0054		0048		0108	
0	0.2	+.1632 0040	0		0019 +.0033				+.0032	+.0068	0029 +.0167	0043 +.0250	0050 +.0302	
		.0040 R _V →			+.1959				├ ॅ	11.0000	1.0101	1.0230	1.0302	1.0020
		R _X R _X	0671		+.0454			+.1061						
	1,0	0671	0	0	0	0	0	0	0	0	0	0	0	0
3/4	0.8		+.0181		0026	0058	0071		+.0036	000 8	0051	0085	0106	0113
1 11	0.6		+.0301		0048 0055	0097	0115		+.0060	0030	0094	0150 0158	0185 0188	0196 0198
% %	0.2	+.1479	+.0155	+.0017		0035	0033		+.0031	0011	0027	0027	0023	0021
9	0	0155	0		+.0060			+.0101	0	+.0138	+.0299	+.0417	+.0484	+.0505
		Ry —►			+.2734									
		R _x R _y			+.0716			+.1102	0	0	0	0	0	
-	0.8	0764 +.1300	+ 0177	0 +.0016	0 0043	0 0060	0062		+.0035	0028	0086	0127		0158
11	0.6		+.0289				0098		+.0058	0054	0149	0213	0248	0259
	0.4				0076	0089	0086	0084	+.0056	0064	0152	0203	0228	0235
q/p	0.2	+.1417	+.0137	0003		0022	0015			0016	0019		+.0003	
		0149	0	+.0042	+.0081	+.0104	+.0115		<u> </u>	+.0211	+.0403	+.0519	+.0576	+.0593
-	<u> </u>	R _X R _y	0743		+.0971		+.1058							
_~	1.0	0743	0	0	0	0	0	0	0	0	0	0	0	0
3/2	0.8	+.1257	+.0145	0025	0055	0052	0046	0044	+.0029	0067	0134	0169	0183	0187
"	0.6		+.0232	0046			0072	0069		0118	0223	0273	0292	0297
	0.4	+.2503	+.0101	0052 0022	0082 0018	0071	0061 2000.+	+ 0004	+.0044	0123	0207	0240 +.0017	+.0026	0253 +.0029
9/p	0.2	0018	0		+.0106				0	+.0339		+.0606	+ 0631	+.0637
	 	Ry	0018		+.3754	+.3960	+.3994	+.3997						
Y	inged		T								M _x		10	
H	min		x	i e p			nent = {(tion = {(W	Rx Ry O N	My CONVE	——→ X

Figure 13.--Plate fixed along three edges---Hinged along one edge, moment and reaction coefficients, Load~IV, uniformly varying load.

	1		•	· · -						<u> </u>					
	L.,	,	 _				×					· · · · · · · · · · · · · · · · · · ·	ly .		
<u> </u>	y/t		%→	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
_	1.0		0000	0000 0	0003 0	0001	+.0000	+.0001	+.0001	0	0	0	0	Γ ο	0
>∞	0.8	3	+.0003	+.0000	+.0000	+.0000	0000	0000	0000	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
1 0	0.6	_	1.0145			+.0000			0003					0000	
8	0.4	_	F.0502 F.0819			+.0001					+.0002			0002 0004	0002
"	0	1	+.0437	0	+.0001	+.0003	+.0004	+.0005	+.0006	0		+.0014			+.0028
<u></u>	\vdash	_	Ry Ry	+.0437	+.0148		+.0807		+.1014	ļ					
1 _	1.0	_	0011	0	0	0	0	0	0	0	0	0	0	0	ō
74	0.8	_				+.0001	0001				+.0001			+.0003	+.0003
1 0	0.6		+.0323 +.1011		+.0015	+.0002	0008 0021							0000 0014	-
%	0.8	_				0005	0025							0027	
"	0		0308	0		+.0009			+.0019	0	+.0019	+.0047	+.0071	+.0088	+.0093
<u> </u>	┼	_	Ry → Ry Ry	+.0308	+.0345 0134	+.1011 0042				 					
١ ـ	1.0	_	0060	0	0	0	0	0	0	0	0	0	0	0	0
\% 8	0.8	_				+.0003			0014					+.0005	
"	0.6	_	+.0558 +.1389			+ .0002 0006	0020		0037				0004		0009 0044
8	0.2		1.1458		+.0031	0012	0034	0043	0046		0002	0023	0038		0050
1	0	_	1.0155	0		+.0016				0	+.0033	+.0081	+.0122	+.0149	+.0158
	┼		Ry → Ry	+.0155 0143	+.0549 0221	+.1390		+.2183	+.2269						
. ~	1.0		0143	0	0	0	0	0	0	0	0	0	0	0	0
~~	0.1					+.0002	0013				+.0007			0001	0002
"_	0.6			+.0163		0003 0016			0033				0020	0028 0073	0078
8	0.2	2 1	1415	+.0119	+.0022	0019	0034	0039	0040	+.0024	0008	0032	0046	0054	
\	<u> </u>		F.0064	0	+.0011	+.0026				0	+.0056	+.0129	+.0187	+.0222	+.0234
-	+		R _V R _V	0276	0216	+.1762									
	1.0) [-	0276	0	0	0	0	0	0	0	0	0	0	0	0
₹ **	0.8			+.0072		0006 0020	0022				+.0004 0003		0020 0060	0028 0077	0031
l u	0.4	_		+.0176			0056				0022	0070	0102	0119	0125
8	0.2	\rightarrow	1319		+.0003		0027			+.0022	0020		0044	0043	
-	-0		R _v	+.0010		+.0043				0	+.0106	+.0214	+.0285	+.0323	+.0335
†			R _y	0329			+.0321		+ 0376						
\ _	1.0		.0329	0	0	0	0	0	0	0	0	0	0	0	0
	0.6		.0341	+.0073	+.0013	0015 0033		0025 0045	0025	+.0015	0001 0018	0021 0060	0038 0091	0048 0107	0051 0113
1	0.4		1623		0002			0049	0048	+.0032	0044	0097	0127	0141	0145
8	0.2			f .0093	0011	0024		0014		+.0019	0028		0035	0031 0371	0029
1	-	_	.0040 R _y →	+.0040	+.0031	+.0055				0	+.0155	+.0275	T.U341	+.0371	7.0380
		Ŕ	R _y	0339	+.0100	+.0313	+.0354	+.0354	+.0351						
3%	1.0		0339	0 +.0063	0006	0 0021	0 0020	0017	0016	+.0013	0 - 0015	0 0042	0 0057	0 0063	0 0065
	0.8		0878	+.0063	0019	0021	0020	0032	0016	+.0022	0013	0095	0037	0129	0131
"	0.4	1	⊦.1577	+.0123	0034	0048	0041	0036	0034	+.0025	0080	0129	0147	0153	0154
%	0.2	_	F.0169	+ .0065	0021 +.0047	0017 +.0069	+ 0077	0006 +.0080	0005 +.0080	+.0013	0035 +.0235	0032 +.0346	+ 0386	+.0019	+.0402
	-		₹ _y →	+.0169	+.2361	+.2888		+.3007	+.3008	<u> </u>					.0702
	Hinged Moment = (Coefficient)(pb ²) Reaction = (Coefficient)(pb)												- x		
1	<i></i>	× × × × × × × × × × × × × × × × × × ×											نہ VE 31 61	/ My I CONVE	NTION

Figure 14.—Plate fixed along three edges—Hinged along one edge, moment and reaction coefficients, Load V, 2/3 uniformly varying load.

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		 -			N	1 _x	 -				N	/l _y	<u>-</u>	
	У/ _b	x /₀ →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
		R _x R _y				0000					<u> </u>			
%	0.8	+.0000 0000	0 0000	+ 0000	+.0000	+.0000	0000	0000	0	+.0000	0	0	0	0
11	0.6	0001		+.0000		0000	0000		+.0000			+.0000		
مي ا	0.4	+.0040		+.0001		0001	+	0001	+.0000	+.0000		+.0000		
8	0.2	+.0459	+.0017		+.0000	0005				+.0001	0001			0002
	ا	Ry -	+.0369		+.0505					+.0005	1+.0012	+.0018	+.0022	+.0023
		R _x R _y	0000	0003	0002	0001		+.0000						
74	0.8	0000 0003	+.0000	+ 0000	+.0000	0000	0000	- 0000	0	+.0000	0	0	0	0
-	0.6	+.0004			+.0001	0001				+.0000		+.0000	+.0000	
	0.4			+.0007		0004	0006	0007	+.0003	+.0002	+.0001	+.0001	+.0000	+.0000
%	0.2	+.0693	+.0040		+.0003	0012				+.0001		0012		
1	<u> </u>	R _v	+.0297		+.0869				0	+.0014	1+.0032	+.0046	+.0055	+.0058
		R _x R _y	0007	0019	0009	+.0000	+.0007							
3%	0.8	0007	0	+.0002	0	0	0	0	0	0	0	0	0	0
1	0.6	+.0002	+.0010	+.0002	+.0001	0001	0001	- 0006	+ 0002	+.0001	+.0001	+.0001	+.0002	+.0002
"	0.4	+.0216		+.0012	0000	0009				+.0003	0000			0005
%	0.2			+.0010		0016	0018		+.0010		0014			0029
1	<u> </u>	+.0228 R _y →	0 +.0228		+.0009		+.0015		<u> </u>	+.0020	+.0044	+.0063	+.0075	+.0078
		R _x R _y	0020		0012		+.0023	+.0028						
2/2	1.0	0020	0	0	0	0	0	0	0	0	0	0	0	0
1	0.8				+.0001	0002 0005	0003 0008	0004	+ 0004	+.0001	+.0001	+.0001	+.0001	+.0001 0001
۾ ا	0.4				0003		0014	0015	+.0007	+.0002	0004	0009	0012	0013
8	0.2				0013				+.0010					0033
1	<u> </u>	+.0203 R _y >	+.0203		+.0013				0	+.0031	+.0063	+.0085	+.0097	+.0101
_		R _x R _y	0044		+.0004									
3,4	1.0	0044	0	0	0	0	0	0	0	0	0	0	0	0
į į	0.8				0000 0002		0005 0009	0005	+.0002	+.0001	0000 0002			0003 0010
"	0.4					0012	0012			0002	0011			0023
9%	0.2		+.0041	0005		0012	0010		+.0008	0015				0032
	<u> </u>	+.0216 R _v →	+.0216	+.0010	+.0018		+.0024		0	+.0052	+.0091	+.0111	+.0121	+.0124
		R _x R _y	0054		+.0024			+.0056			-			
l	1.0	0054	0	0	0	0	0	0	0	0	0	0	0	0
1 11	0.8	+.0045		+.0003			0004 0007	0004 00 0 7	+.0002	+.0001 0000		0005 - 0011 -		0007 0016
اما	0.4		+.0035	0000		0010	0010	- 0000.		0006				0027
٩/ _p	0.2		+.0033	0009						0021		0030		
1	0	+.0261 R _v →	0 + 0261	+.0014	+.0022		+.0026 +.1619		0	+.0070	+.0108]	+.0124	+.0131]	+.0133
					+.0045			+.0051						
~2	1.0	0058	0	0	0	0	0	0	0	0	0	0	0	0
, w	0.8	+.0044		0000			0003 0005					0008 - 0017 -		
"	0.4	+.0232	+.0027	0007	0010	0008	0007	0006	+.0005	0014	0024	0028	0029	0029
9/p	0.2		+.0021	0011				0006				0028		0027
}	0	+.0360 R, →	+.0360		+.0025				0	+.0095	+.0125	+.0135	r.UI36]	+.0136
		<u> </u>	.5000											
H	Hinged Moment = (Coefficient)(pb ²) Reaction = (Coefficient)(pb)													— → X
<i></i>	POSITIVE SIGN CONVENTIC												NTION	

 $\begin{tabular}{ll} \textbf{Figure 15.--Plate fixed along three edges---Hinged along one edge, moment and reaction coefficients, Load VI, 1/3 uniformly varying load. \end{tabular}$

					N	1 _x			<u> </u>	· · · · · ·	٨	n _y		
1	y/b	¾ _a →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
		R _x R _y	0000			0000		0000		<u> </u>	<u> </u>		L 0.0	
<u>~</u>	1.0	0000	0000	0000	0	0	0	0	0	0	0	0	0	0
l ii	0.8	0000			+.0000		+.0000			0000 +.0000	+ 0000	+.0000	+.0000	+.0000
	0.4	0000			+.0000			0000	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
1%	0.2	+.0119	+.0004		+.0000			0002	+.0001	+.0000	+.0000	+.0000	+.0000	+.0000
	0	Ry →	+.0237		+.0002				0	+.0003	+.0008	+.0012	+.0014	+.0014
	1	R _x R _y	0000	0000		0000		0000						
74	0.8	0000	0	0	0	0	0	0	0	0	0	0	0	0
1	0.6	0001			+.0000			- 0000	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
"	0.4	+.0010			+.0000		0001	0001	+.0000	0000.+	+.0000	+.0000	+.0000	+.0000
1%	0.2	+.0176			0001		0003	0004	+.0002	+.0001	0001	0002	0002	0002
	-0	+.0214 R _v →	+.0214		+.0003				0	1+.0008	+.0016	+.0021	+.0024	+.0025
	-	R _x R _y	0001	0003		0000								
	1.0	1000.	0	0	0	0	0	0	0	0	Ö	0	0	0
3%	0.8	+.0004	+.0000	+.0000	+.0000	0000	0000	0000	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
"	0.4	+.0028	+.0005	+.0002	+.0000	0001	0002	0002	+.0000	+.0001	+.0001	+.0000	- 0000	- 0000
1 %	0.2	+.0219				0004	0004	0004	+.0003	+.0000	0002	0004	0005	0006
1	-	+.0196	0		+.0003				0	+.0008	+.0016	+.0022	+.0026	+.0027
<u> </u>	+	R _x R _y	+.0196	0007	+.0685	+.0001								
.~	1.0	0003	0	0	0	0	0	0	0	0	0	0	0	-
1/2	0.8		+.0001	+.0001	+.0000	0000	0001	0001	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000
ti -	0.6	+.0010	+.0003	+.0002	0000	- 0001	0001	0002	+.0001	+.0001	+.0000 0000		+.0000	
1%	0.2	+.0214	+.0012		0003					0001	0004			0002 0007
1 "	0	+.0202	0		+.0004	+.0006	+.0006	+.0006	0		+.0022			+.0032
<u> </u>	<u> </u>	$R_{x} \rightarrow R_{y}$			+.0740									
	1.0	0008	000 8	0	+.0000. 0	0	0	0	0	0	0 1	0	0 1	 -
1 %	0.8		+.0002		0000	0001	0001			+.0000		0000		0000
1	0.6		+.0004			0001				+.0000	-	0001	0001	0001
1 %	0.4	+.0039	+.0007		0001 0003	0002 0003					0001 0006	0003 0007	0003 0007	0004 0007
0	0	+.0233	0		+.0006				0		+.0029		+.0036	
		R _y			+.0791									
	1.0	R _x R _y	0009 0	0005 0	+.0003 0	+.0007	+.0009	+.0009	0	0	0	0 1	0 1	-
-	0.8		+.0002			0001	-	-		+.0000			0001	
11	0.6		+.0004			0001				+.0000		0002	0002	0002
1%	0.4	+.0037	+.0007			0002				0000		0004		0004
6	0.2	+.0270	0		+.0007				0	+.0004	0006 +.0033	0007 +.0036		0007 +.0038
		R _y →		+.0742	+.0811	+.0824	+.0826	+.0827			1			
	<u> </u>	R _x R _y			+.0007		+.0008	+.0008 0	•		0 1			
%	0.8	0010 +.0007	+.0002	0 0000	0 0001	0001			+.0000	0 0000	0 0001	0001	000 I	000 I
] "	0.6	+.0016	+.0004	0000	0001		0001	0001	+.0001		0002	0003	0003	0003
	0.4		+.0005	0001	0002	0001	0001	0001	+.0001	0002	0004	0004	0005	0005
9/0	0.2	+.0232	+.0005	0003 +.0006	0002 +.0007	+.0008		0001 +.0008	+.0001	+.0006	0007 +.0036	0006 +.0038	+.0039	+ 0039
L	Ľ	Ry					+.0826						.0000]	.000
Y												Y	***	
*	linged			¥	_		nent = ((tion = ((M _x	R _x	/M _y	→ X
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	//////////////////////////////////////		^	, _, T	 ← p						POSITI	VE SIGN	CONVE	NTION

Figure 16.—Plate fixed along three edges—Hinged along one edge, moment and reaction coefficients, Load VII, 1/6 uniformly varying load.

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					N	1 _x					٨	Иy		
	y/b	% ₀ →	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
		R _X R _y			+4.9866					*	•		·	
׺	1.0	+.6697	+.2000	+.2000	+.2000	+.2000	+.2000	+.2000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
11	0.8	-3.0788 0357	1142	0481	0003	+.0322	+.0509	+.0571	0228	0080	+.0046	+.0142	+.0201	+.0221
	0.6	+.0069								0017 0001				
8	0.2	+.0007	+.0000	+.0000	0000	0000	0000	0000	+.0000	+.0000	0000	0000	0000	- 0000
"	0	+.0001	0	+.0000	+.0000	+.0000	+.0000	+.0000	0			+.0000		
		Ry	+.0001		0003							•		
İ		R _x R _y			+4.6774									
₹	0.8	+1.5770 -5.4606	+.2000	+.2000	+.2000	+.2000	+.2000	+.2000		+1.0000				
	0.6	4874			0097					0052 0144				
H	0.4	+.0172			0039					0036				0103
8	0.2	+.0192			0009					0006				
١	0	+.0045	0		+.0000				0			+.0001		
			+.0045											
			+2.4931											
3/8	1.0		+.2000	+.2000	+.2000	+.2000	+.2000	+.2000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
	0.8	-6.6847 -1.2696			0084					+.0241		+.2060		
u.	0.4	1792			0119							0210		
و/p	0.2		0126		0057					0054		0144		
6	0	+.0436	0		0012				0			0130		0210
		Ry			+.0472									
			+3.2446											
1/2	1.0									+1.0000				
	0.8		5260 3238							+.0687				
11	0.6				0128					0243				
a/p	0.2				0113					0143				
"	0	+.1191	0		0061			0187	0			0611		0937
		Ry →			+.0313									
			+4.1774											
3/4	1.0				+.2000					+1.0000				
	0.8	-2.3733			+.0607					+.0025				
"	0.4	7580			+.0102					0269				
₀⁄₀	0.2	+.2313			0124					0344				
0	0	+.2415	0		0230				0	0292	1151	2041	2651	2863
		Ry			2583									
			+4.5863						41.0000	+10000	+1.00001	+1 00001	+1 00001	+1 0000
_	0.8		+.2000 4636							+1.0000				
-	0.8		4636 4029							+.2639				
	0.4	7966								0178				
۹% ا	0.2	+.2918			0086					0504				
	0	+.3067	0	0131	0404	0631	0762	0804	0	0657	2021	3153	3811	4019
			+.3067											
		R _X R _y	+4.8170	+2.5171	+1.5692	+1.4276	+1.4268	+1.4337	+10000	+1 000 aT	+1.00001	+1 0000	+100001	+1 0000C
3/2	1.0		+.2000 3365							+1.0000				
	0.8	-2 0746	- 3261	+ 0566	+ 1203	+ 1114	+ 0985	+ 0939	0652	+.1391	+.2901	+.3639	+.3932	+.4006
11_	0.4	7384	-,1975	+.0076	+.0543	+.0475	+.0367	+.0328	0395	+.0103	+.0656	+.0969	+.1096	+.1127
a/b	0.2	+.3738	0607	0163	0110	0213	0289	0313	0121	0757	255	1550	1690	1729
	0	+.3532	0	0314	0664	0846	0917	0934	0	1570	3320	4231	4584	4670
		Ry →	+.3532	3898	-1.1608	-1.4315	-1.4935	-1.5015						
¥												~		

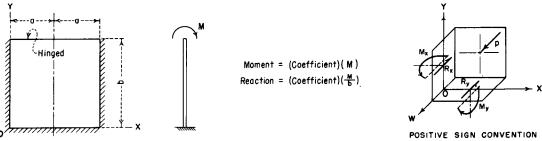


Figure 17.—Plate fixed along three edges—Hinged along one edge, moment and reaction coefficients, Load VIII, moment at hinged edge.

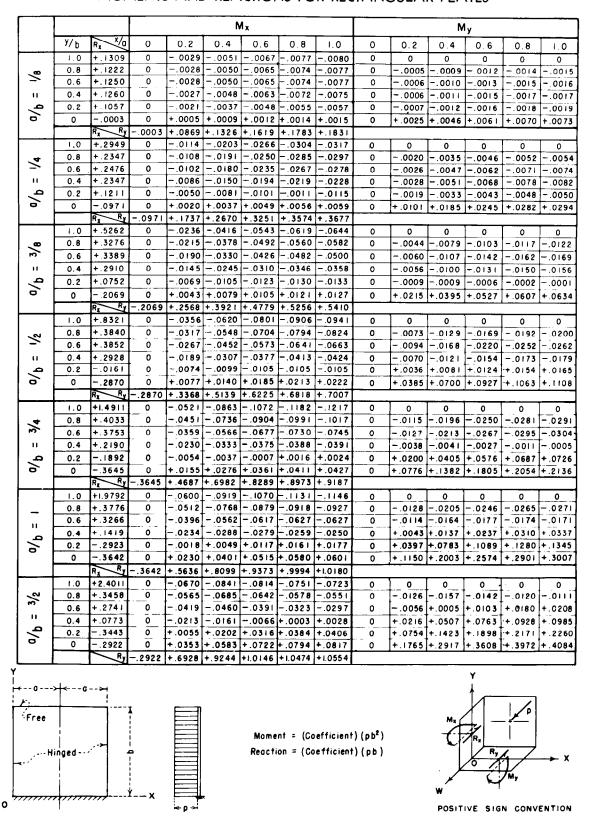


FIGURE 18.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load I, uniform load.

					٨	1 _x		•	My							
	У/Ь	R _X X/ _Q	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0		
	1.0	0005	0	- 0002	- 0003	0004	0005	0005	0	0.2	0.4	0.0	0.8	0		
-%	0.8	+.0104	0	0005	0009	0012	0015	0014	0	+.0000	+.0001	+.0001	+.0001	<u> </u>		
	0.6	+.0949	0	0020	0034	0045	0051	0053	0	0005	0009	0012	0013	0014		
"	0.4	+.1251	0	0026	0045	0059 0048	0067	0070	0	0007	0012	0016	0018	0019		
٩/ _p	0.2	+.0000	0	+.0005	+.0009	+.0012	+.0014	0056 +.0015	0	+.0007	+.0046	+.0016	+.0070			
		Rx Ry	+.0000	+.0858	+.1332	+.1616	+ 1779	+.1830	l	1.0023	17.0046	1 + .0001	14.0070	1+.0073		
	1.0	0012	0	0020	0039	0053	0062	0065	0	0	0	0	0	0		
' 4	0.8	+.0413	0	0031	0058	0078	0091	0095	0	+.0001	+.0001	+.0001	+.0001	+.0001		
1	0.6	+.1802	0	0063 0072	0110	0142 0157	0161	0167	0	0023	0041	0055	0063	0066		
ما	0.4	+.1272		0046	0075	0091	0100	0183	0	0032	0058 0039	0077 0051	0088 0058	0092 0060		
0/p	0	0821	0	+	+.0034	+.0045		+.0054	0	+.0094	+.0170	+.0225	+.0258			
		R _X R _Y	0821	+.1677	+.2555	+.3095	+.3389	+.3483		1			1.0200	10200		
	1.0	+.0280	0	0060	0113	0154	0179	8810	0	0	0	0	0	0		
3/8	0.8	+.0821	0	0073	0134	0178	0206	0215	0	0007	0014	0019	0023	0024		
11	0.6	+.2388	0	0108 0109	0183 0177	0232 0218	- 0260	0269	0	0051	0092	0121	0139	0145		
ا م	0.2	+.1088	0	0060	0089	0101	0105	0246	0	0064	0115 0041	0152	0174 0052	0053		
9/p	0	1488	0	+.0035	+.0063	+.0083	+.0095	+.0099	0	+.0173	+.0314	+.0416	+.0477	+.0497		
1 1		R _X R _X	1488	+.2333	+.3475	+.4166	+.4538		,	10.10		1.0 7.0	1.0117	,		
	1.0	+.1033	0	0102	0190	0255	0296	0309	0	0	0	0	0	0		
%	0.8	+.1094	0	0112	- 0201	0265	- 0302	0315	0	0019	0037	0053	0063	0067		
1 1	0.6	+ 2693	0	0143	0235	0291	0321	0330	0	0081	0144	0188	0215	0224		
"_	0.4	+.2881	0	0133 0065	0205 0084	0241 0085	0257 0082	0262 0081	0	0090 0012	0157 0009	+.0001	+.0010	0237 +.0013		
٩/ _p	0.2	1793	0	+.0055	+.0098		+.0146	+.0151	0	+.0276	+.0492	+.0642	+.0729	+.0757		
		R _X R _Y	1793	+.2895	+.4246	+.5005	+.5392	+.5512								
	1.0	+.3075	0	0158	0281	0363	- 0409	0423	0	0	0	0	0	0		
3%	0.8	+.1194	0	0161	0273	0341	0376	0386	0	0039	0076	0106	0124	0130		
"	0.6	+.2754	0	0185 0158	0277 0211	0320 0223	0338 0223	0343 0222	0	0121	0206	0260 0207	0290	0299		
1 1	0.2	+.0037	0	0060	0052	0031	0016	0011	0	+.0044	0175 + 0111	+.0175	+.0219	+.0234		
8	0	1906	0	+.0096			+.0235	+.0243	0	+.0482		+.1052	+.1176	+.1216		
		RX	1906	+.3764	+.5274	+.6009	+.6352	+.6452		ر						
	1.0	+.4673	0	0184	0304	0366	0392	0399	0	0	0	0	0	0		
-	0.8	+.1073	0	0184	0286	0329	0344	0346	0	- 0048	0091	0120	0137	0142		
"	0.6	+.2619	0	0203 0165	0273 0189	0288	0287	0285 0154	0	0140 0107	0220	0261 0140	0277	0282 0122		
8	0.2	0251	0	0047			+.0047	+.0054	0	-		+.0364	+.0438	+.0463		
•	0	1712	0	+.0133	+.0220	+.0273	+.0301	+.0310	0		+.1101		+.1507	+.1552		
L		RY Rx	1712	+.4372	+.5845	+.6467	+.6731	+.6805								
	1.0	+.6045	0	0213	0285	0279	0257	0246	0	0	0	0	0	0		
3/2	0.8	+.0865	0	0209	0258	0241	0214	0204		0057	0092	0102	0102	0100 0170		
"	0.6	+.2515	0		0230 0131		-	0154 0043	0	0156 0087		+.0020				
ا م	0.2	0201	0	+	+.0053		_	+.0140	0	-	+.0477		+.0750			
^ [0	1167			+.0295				0				+.1888	+.1929		
		P.	1167	+.5160	+.6384	+.6754	+.6867	+.6891								
Free Mament = (Coefficient) (pb²) Reaction = (Coefficient) (pb) W																
	•			⊷-p->-i							POSITI	VE SIGN	N CONVE	NTION		

 $\begin{tabular}{ll} Figure 19.--Plate fixed along one edge--Hinged along two opposite edges, moment and reaction coefficients, Load II, 2/3 \\ uniform load. \\ \end{tabular}$

	T				A.	1 _x										
1	y/b	R _x X/ _Q	 _ _	T = =				·	My							
				0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0		
1 _	0.8	+.0001	0	0000		0000				0	0	0	0	0		
~ %	0.6	+.0011	 	0001	0003	000 I 0003	000 I		0				+ 0000	_		
11	0.4	+.0304	0	0007	0013	0017	0020	0021	0	+.0000				+.0001		
% 9%	0.2	+.0966	0	0017	0030		0043	0044	1	0007	0002 0013	0003 0017		0003		
0	0	+ .0059	0	+.0005	+.0008	+.0011	+.0013		0	+.0023	+.0042		+.0063			
ļ		R _x R _y	+.0059	+.0830	+.1263	+.1528			 	1	1.0042	11.0000	11.0003	14.0088		
}	1.0	0024	0	0002	0004	0006	0007	0008	0	0	0	0	0	0		
74	0.8	+ 0019	0	0004	0007	0010	0012	0012	0	+.0001	+.0002	+.0002	+.0003	+.0003		
-	0.6	+.0099	0	0009	0016	0022	0025	0027	0	+.0000	+.0001	+.0001	+.0001	+.0001		
1	0.4	+.0614	0	0021	0036	0046	0052	0054	0	0008	0015		0024	0025		
\ 2	0.2	+.1193 0327	0	+.0013	0047 +.0023	0055	0059	0060	0	0024	0042			0064		
1	-	R Ry	0327	+.1414	+.2062	+.0030	+.0034	+ .2659	0	+.0065	+.0115	+.0150	+.0170	+.0177		
	1.0	0026	0	0009	0017	0023	0027	0028	0	0	0	0	0			
_00	0.8	+.0080	0	0011	0021	0028	0033	0034	Ö		+.0001	+.0002	+.0002	+.0002		
3/8	0.6	+.0207	0	0017	0031	0041	0047	0049	0	0003	0005	0008	0009	0010		
n i	0.4	+.0759	0	0030	0050	0060	0066	0067	0	0018	0033	0045	0052	0054		
9/p	0.2	+.1236	0	0039	0052	0054	0054	0053	0	0034	0057	0071	0079	0081		
0	0	0496	0	+.0019	+.0033	+.0043	+.0048	+.0050	0	+.0094	+.0166	+.0214	+.0242	+.0250		
		R _X	0496	+.1783	+.2458	+.2806	+.2970	+.3018								
	1.0	+.0058	0	0015	0029	0039	0046	0048	0	0	0	0	0	0		
2/	0.8	+.0128	0	0017	0032	0043	0050	0052	0	0001	0002	0003	0004	0004		
1 1	0.6	+.0263	0	0023	0040	0052	0058	0060	0	0007	0013	0019	0023	0024		
"	0.4	+.0788	0	0036 0042	0054	0062	0065	0065	0	0026	0047	0062	0071	0073		
%	0.2	0477	0		+.0048 +.0045	+.0056	+.0062	0040 +.0063	0	0039	0061	0070	0073	0074		
	<u> </u>	R _x Ry	0477				+.3166	+.3200		+.0131	+.0223	+.0279	+.0308	+.0317		
	1.0	+.0340	0	0024	0043	0056	0064	0066	0	0	0	0	0 1	-		
	0.8	+.0148	0	0025	0043	0055	0061	0062	0	0003	0008	0012	0015	0016		
3/4	0.6	+.0267	0	0030	0047	0055	0058	0058	ō	0012	0024	0034	0040	0042		
] 11	0.4	+.0752	0	0043	0054	0054	0052	0051	0	0036	0059	0073	0079	0080		
8	0.2	+.1134	0	0043	0037	0028	0022	0020	0	0042	0051	0046	0040	0038		
0	0	0337	0	+.0039	+ .0062	+.0074	+.0080	+.0081	0	+.0196	+.0309	+.0369	+.0398	+.0406		
		R _X R _Y	- 0337			+.3256		+.3338								
1	1.0	+.0569	0	0027	0046		0060	0061	0	0	0	0	0	0		
-	0.8	+.0131	0		0045		0054	0054	0			0015	0018	0019		
1 "	0.6	+.0233	0			0048	0047	0046	0		0028	0036	0041	0042		
1%	0.2	+.1190	0	0045 0039	0048 0026	0042	0038	0036	0	0041	0061	0067	0068	0067		
°	0.2	0171		+.0050		0015 - +.0085	+.0009 +.0091	+.0007 +.0092	0		0034	0017		0001		
1 t	_	R _X Ry				+.3328		+.3376	<u> </u>	+.0248	+.0369	+.0426	+.0453	+.0461		
 	1.0	+.0765	0	0032				0036	0	0 1	0	0	0 1			
3/2	0.8	+.0102	ō				+	0030	0	0005				0014		
I L	0.6	+.0180	0			0032							0028			
] " [0.4	+.0688	0	0044	0036	0025	0019	0017	0	0046	0053	0047	0040	0037		
중 [0.2	+.1465	0			+.0000		+.0007	0	0030		$\overline{}$		+.0046		
1 2 1	٥	+.0106				+.0098		+.0103	0	+.0321	+.0439	+.0488	+ 0510	+.0516		
oxdot		- Ry	+.0106	+.2977	+.3305	+.3360	+.3372	+.3374								
Free Hir	a							efficient efficient			Mx (R ₁	\rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \rangle \ran	X		
* -*	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	,,,,¥	-x Y	- p -							WPOSITIV	VE SIGN	M,	NTION		

FIGURE 20.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load III, 1/3 uniform load.

			Mx							My							
ľ	y/b	R _X X/0	0 0.2 0.4 0.6 0.8 1.0						0	0.2	0.4	0.6	0.8	1.0			
	1.0	+.0094	0	0003	0006	0008	0010	0010	0	0	0	0	0	0			
<u>«</u>	0.8	+ 0253	0	0006	0011	0015	0017	0017	0	0001	0002	0002	0002	0003			
-	0.6	+.0500	0	0011	0020	0026	0030	0031	0	0002	0004	0005	0006	0006			
= q/o	0.4	+.0758	0	0016 0015	0028	00 3 7	0042	0044	0	0004	0007	0009 0013	0011	0011			
	0.2	+.0833	0	+.0004			+.0012	+.0012	0	+.0021	+.0039	+.0051	+.0058				
	<u> </u>	R _x R _y	+.0043	+.0775	+.1180	+.1417	+.1551	+.1591		1.0021			1				
	1.0	+.0249	0	0022	0040	0054	0062	0065	0	0	0	0	0	0			
_ t	0.8	+.0548	0	0028	0051	0067	0077	0081	0	0004	0006	0008	0010	0010			
₹	0.6	+.1014	0	0040	0070	0091	0104	0108	0	0012	0021	0028	0032	0033			
11	0.4	+.1388	0	0046		0100	0113	0117	0	0019	0034	0046	0052	0055			
0/ _ه	0.2	+.1003	0	0033		0065	0071	0072	0	0018	0031	0041	0046	0048			
٥` ا	0	0513	0		+.0025		+.0038	+.0040	0]+.0071	+.0127	+.0167	+.0191	+.0199			
	$\overline{}$	R _X R _Y	0513	+.1404	+.2080		+.2688	+.2754		1 0		0	0	0			
- }	1.0	+.0631	0	0054 0060	0099 0108	0133	0154	0160 0170	0	0011	0020	0026	0030	0031			
3%	0.8	+.0852	0	0071	0122	0156	0176	0183	0	 	0050	0066	0076	0079			
- 1	0.6	+.1388 +.1701	0	0070	0116	0143	₹.0158	0162	0	0038		0091	0104	0108			
ا ما	0.4	+.0890	0	0044	0064	0071	0074	0074	0	0021	0033	0040	0043	0044			
% %	0.2	0960	0		+.0045		+ 0068	+.0071	0	+.0124		+.0296	+.0339	+.0353			
ŀ		R _x R _y	0960	+.1886	+ 2717	+.3203		+.3538									
	1.0	+.1322	0	0087	0159	0210	0241	0252	0	0	0	0	0	0			
ا ہے	0.8	+.1049	0	0090	0159	0207	0235	0244	0	0021	0038	0050	0058	0061			
~ <u>`</u>	0.6	+.1584	0	0096	0161	0201	0224	0231	0	0045	0080	0106	0121	0127			
- 11 [0.4	+.1757	0	0087	0136	0161	0173	0176	0	0053	0092	0119	0134	0139			
9/p	0.2	+.0595	0	0047	0060		0058	0057	0	0011	0010	0004	+.0002	+.0005			
9 [0	1169	0		+.0070		+.0103	+.0107	0	+.0198	+.0351	+.0455	+.0515	+.0535			
		R _x R _y		_	+.3261		+.4050	+ 4130		1 _							
	1.0	+.3017	0	0132		0292	0326	0337 0299	0	0036	0065	0086	0098	0 0102			
3/4	0.8	+.1125	0		0214	0265 0227	0291	0246	0	0066	0114	- 0145	0162	0168			
+	0.6	+.1595	0		0142	0151	0152	0152	0	0060	0093	-0109	0114	0115			
"_	0.4	+.0115	0	0043		0021	0011	0007	0		+.0080	+.0128	+.0161	+.0172			
9/6	0.2	1259	0		+.0119	+.0151	+.0168	+.0174	0	+.0348	+.0594	+.0753	+.0842	+.0871			
_ }		R _X R _y	1259	+.2925	+.3991	+.4504	+.4744	+.4816									
	1.0	+.4312	0	0154	0247	0293	0312	0317	0	0	0	0	0	0			
_ 1	0.8	+.1046	0	~.0147	0224	0256	0268	0270	0	0043	0073	0093	0103	 			
- 1	0.6	+.1468	0	0139	0191	0205	0205	0204	0	0073	0115	0135	+	0145			
"	0.4	+.1380	0	0109	0125	0117	0107	0103	0	0050	0059	0048	- 0035				
0⁄ _ه	0.2	0109	0	0032			+.0036	+.0041	0		+.0186	+.0275	+	+.0350			
	0_	1123	0		+.0159			+.0226	0	+.0482	+.0797	+.0990	1+.1094	+.1128			
		R _X R _Y	-1123	+.3359	ļ		+.5035		 	1 0	0	0	0	0			
	1.0	+.5418	0	0175 0164	+	+		0197	0	0048	- 0069		0070				
3/2	0.8	+.0931	0		0160	 			ŏ	+	0086		 	0050			
- 11	0.6	+.1335	0		0083				0				+.0129	+.0144			
	0.2	0072			+.0043				0	+.0178	+.0365		+.0579	+.0604			
0/p	0	0724	0		+.0216			+.0285	0	+.0694	+.1079	+.1289	+.1394	+.1426			
		Ry	0724	+.3914	+.4785	+.5060	+.5149	+.5169	<u> </u>								
Free H	inged	>					•	oefficien oefficien			Mx (R _x	P My				
mm	777777	<i>,,,,</i> ,	. ¥x	¥ 0 ×							W POSIT	TIVE SI	M, GN CON	·			

Figure 21.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load IV, uniformly varying load.

$8/_1 = 9/_0$ $8/_1 = 9/_0$	y/b i.0 o.8 o.6 o.4 o.2 o	Rx X/0 0004 +.0007 +.0152 +.0508 +.0721 +.0065	0 0 0 0	0.2 0000 0001	+	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1			
a/b = 1/4 $a/b = 1/6$	0.8 0.6 0.4 0.2 0	+.0007 +.0152 +.0508 +.0721	0	0.8 +.0007 00001000100020002 0 +.0000 +.0000 +.0001													
a/b = 1/4 $a/b = 1/6$	0.6 0.4 0.2 0	+.0152 +.0508 +.0721	0	+		+	+		0	0			0	1.0			
0/b = 1/4 0/b	0.4 0.2 0	+.0508 +.0721	1	1 - OOO4		+			+	+	+	 	+.0001				
0/b = 1/4 0/b	0.2	+.0721	I	0011	0019	+	0011	0012		- 0000	0001	0001	0001				
a/b = 1/4	1.0	+.0065	0	0013	0023	+	0034	-	+	0003	0005	0006	0007				
= q/ ₀			0	+.0004	+.0007	+	+ 0010	+	+	+.0019	+.0035		0013 +.0052	+ 0055			
= q/ ₀		R _x R _y	+.0065	+	+4100	+.1316	+.1436	+.1476					17.0032	11.0033			
= q/ ₀		0025	0	0005	0009	+	0015	0015		0	0	0	0	0			
0/p	0.6	+.0067	0	0008 0017	0014 0030		0023 0045	0024		+.0001	+.0002			+.0003			
-	0.4	+.0915	0	0028	0048		0049	0047	0	0003	0006 0024	0008 0032	0010	0010			
-	0.2	+.0885	0	0026	0041		0053	0054		0016	0024	0032	0037 0043	0038 0045			
	<u> </u>	0314	0	+.0011	+.0020	+.0026	+.0030		0	+.0057	+.0102		+.0150	+.0156			
1	<u> </u>	R _X R _Y	0314	_	+.1807		+.2282										
	0.8	+.0015	0	0016 0019	0030	0040	0047	0050		0	0	0	0	0			
3/8	0.6	+.0531	0	0030	0036 0052	0048	0056 0076	0059		0000	0001	0001	0001	0002			
u l	0.4	+.1106	0	0041	0066	0081	0088	0079 0090	+	0010	0019 0048	0025 0063	- 0029	0031			
%	0.2	+.0876	0	OD33	0046	0050	0051	0051	0	0023	0038	0047	0072 0052	0075 0053			
0	0	0539	0	+.0018	+.0032	+.0041	+.0047	+.0049	0			+.0206		+.0243			
		R _X R _Y	0539		+.2213	+.2550	+.2718	+.2769									
1 1	1.0	+.0192	0		0051	0069	0080	0084	0	0	0	0	0	0			
U [0.4]+ 1150 0 [- 0049]- 0074 0005 0000 0001																	
0.4 + .1150 000490074008600900091 00037006500840095																	
0.2 +.0760 000360044004200390038 0002300330037003700 00580 0 +.0026 +.0045 +.0058 +.0064 +.0067 0 +.0130 +.0226 +.0288 +.0322 +.0																	
0 - 0500 0 + 0025 + 0045 + 0050 + 0050																	
$\begin{array}{c ccccccccccccccccccccccccccccccccccc$																	
R _x																	
1.0 +.0720 000420076009901110115 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0																	
0.8 +.0287 000430075009401040107 0000800170025003000 0.6 +.0634 000520079009200970098 0002900520068007700 0.4 +.1099 000580075007700750074 000490078009401040104																	
ام	0.2	+.0614	0		0031	0022		0013	0		0005			0103			
9/p	0	0506	0	+.0042	+.0068	+.0084	+ .0092							.0471			
[0506	+.2266	2910	+.3166	+.3271	+ 3300									
' ⊦	1.0	+.1140	+		$\overline{}$	0099	0106	0108	0	0	0	0	0	0			
-		+.0254		\rightarrow	0078			0095						0036			
" -	$\overline{}$	+.1043	0		0078 0067	0082 0061		0080 0053	0					0078			
9/9		+.0590					F.0004							0077			
° [0	0362	0			+.0102								.0565			
		R _X Ry	0362	+.2528 +				+ . 3/3 86									
H		+.1502				0075		0066	0	0	0	0	0	0			
3/2		+.0198 +.0526	-			0065 -				- 0012 -							
- n		+.1036			.0049	0055 - 0034 -		0042 0021	0	0039 0054 -	-		.0051 -	0049			
8		+.0748		0020 +	-		.0028		+			.0139 +		.0174			
o` [0	0083		$\overline{}$.0107 -		0131		+					.0666			
	1	Ry-	0083	2838 +	.3265	+.3366	+.3393	.3398									
Free Hin	nged		_x		• -		nt = (Co on = (Co				M	R _x	M, CONVE	→ X			

FIGURE 22.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load V, 2/3 uniformly varying load.

				·	N	1 _x					N	 Лу			
	у/ь	R _X X/o	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0	
	1.0	+.0000	0	0000	0000	0000	0000	0000	0	0	0	0	0	0	
<u>_</u> e	0.8	0002	0	0000			0000	0000	0	+.0000	+.0000	+.0000	+.0000		
	0.6	+.0001	0	0000		0001	0001	0001	0	+.0000	+.0000		+.0000	+.0000	
"	0.4	+.0395	0	0007	0003	0005	0005	0006 0017	0	0000	0000 0005	0000 0006	0000 0007	0000 0007	
%	0.2	+.0111	0		+.0005		+.0007	+.0008	0	+.0014	+.0025	+.0032	+ 3037	+.0036	
		R _X R _Y	+.0111	+.0616			+.1125	+.1149		1					
	1.0	0008	0	0001	†	0002	0002	0002	0	0	0	0	0	0	
🛂	0.8	+.0004	0	0001	0002		0003	0003	0	+.0000	+.0001	+.0001	+.0001	+.0001	
1 1	0.6	+.0030	0	0002 0006	0005 0010		0007	0008	0	+.0000		+.0000	$\overline{}$	+.0000	
ا ۃ ا	0.4	+.0133	- 0	0012	0017	0013 0020	0015 0021	0015 0022	0	0002	0003 0015	0004 0020	0005 0023	0005 0024	
% %	0	0008	0		+.0011	+.0014	+.0016		0	+.0032	+.0055	+.0070	+.0078	+.0080	
		R _x R _y	0008	+.0923	+.1244	+.1400	+.1472	+.1493							
	1.0	0010	0	0002	+	0006	0007	0008	0	0	0	0	. 0	0	
3%	0.8	+.0020	0	0003	0006	0008	0009	0010	0	+.0000	+.0001	+.0001	+.0001	+.0001	
1 1	0.6	+.0051	0	0005	+	0012	0013	0014	0	0000	0001	0001	0002	0002	
"	0.4	+.0174	0	0008	0014	0017	0019 0019	0019	0	0004	0008 0022	0010	0012	0013	
8	0.2	+.0529	 		+.0014		+.0020	+.0019	0	0013 +.0040	+.0069	0028 +.0088	+.0031	0032 +.0101	
		R _v R _v	0047	+.1091	+.1384		+.1577	+.1593	_	1 .0040	1.0003	1.0000	1.0036	`````	
	1.0	+.0010	0	0004		0011	0012	0013	0	Q	0	0	0	0	
اہا	0.8	+.0033	0	0005	0009	0012	0014	0014	0	0000	0000	0001	0001	0001	
%	0.6	+.0066	0	0006		0014	0016	0017	0	0001	0003	0004	0005	0006 0018	
"															
00009 0 +.0011 +.0018 +.0022 +.0024 +.0024 0 +.0054 +.0089 +.0109 +.011														0031 +.0122	
ľ	R _X R _y 0009 +.1211 +.1488 +.1593 +.1632 +.1642														
	R _R														
.4	1.0 +.0083 000060012001500170018 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
m	0.6 +.0068 000080013001500160016 00003000600080010														
0	11 0.4 +.0168 000120015001500140014 00008001500180020														
9															
\vdash	1.0	+.0142	0	0007	-	0015	0016	0016	0	0	0	0	0	•	
	0.8	+.0034	0	0008		0014	0014	0014	0	0001	0002	0 004	0005	0005	
-	0.6	+.0059	0	0009	0013	0013	0013	0012	0	0003	0007	0009	0010	0011	
1	0.4	+.0152	0	0013			0010	0010	0	0010	0015	0017	0017	0017	
9/p	0.2	+.0554	0	0015		7.000	0005	0004	0	0019	0020	0016	0013	0011	
	0	+.0179	+.0179	+.1462	+.0027	+.0030	+.0032	+.0032	0	+.0095	+.0135	+.0152	+.0159	+.0161	
-	1.0	+.0193	0	0009		0011	0010	0009	0	1 0	0	Ó	0	-	
3/2	0.8	+.0026	-	0009	+	0010	0008	0008	0	0001	0003	0003	0004	0004	
1 1	0.6	+.0046	0	0010	0010	0008	0007		0	0004		0007	-:0007	0007	
"	0.4	+.0135	0	•	0010		0005		0	0011	0014	0012	$\overline{}$	0010	
۵/ _p	0.2	+.0679	<u> </u>		0006	0002	0001	-	0	0018	0011	0004		+.0001	
	0	+.0320	+.0320		+.1662	+.0034	+.0035		0	+.0119	+.0155	T.0168	+.0174	+.0175	
				17.7557	1002		, ., , ,	0, 3							
Free H	inged	, >	T				• •	efficient efficient			Mx.	R) M,		
	1			->-p							POSIT	IVE SIG	N CONVI	ENTION	

Figure 23.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load VI, 1/3 uniformly varying load.

			Γ		N	1 _x			1	·		Лy			
	y/b	Rx X/o	,0	0.2	0.4	0.6	0,8	1.0	0	0 2	0.4	0.6	T 0 0	T	
	1.0	0000	0	0000	0000	0000	0000	0000		102			0.8	1.0	
_%	0.8	0000	0	0000	0000	0000		0000	4		+.0000	+ 0000	0	+.0000	
->	0.6	0000	0	0000	0000	0000	0000	0000	+		+.0000		+.0000		
- 11	0.4	+.0002	0	0000	+	0001	0001	0001	0			+.0000	+.0000		
%	0.2	+.0134	0	0002	 	0004	0004	0004	0	0001	0001	0001	0001	0001	
"		+.0103	0	+.0002				+.0004		+.0008	+.0014	+.0018	+.0020	+.0020	
<u> </u>	1.0	0002	+.0103	+.0459	+.0630	+.0699		+.0750							
	0.8	+.0000	0	0000	0000	0000 0001	0000	0000	<u> </u>	0	0	0	0	0	
74	0.6	+.0007	0	0000	0001	0001		0001 0002	0		+.0000	+.0000	+.0000		
1 0	0.4	+.0015	0	0001	0002	0002	0003	0003	0	0000	0000	0000	0000	0000	
9/p	0.2	+.0124	0	0002	0004	0004	0004	0004	0	0000	0000 0003	0000	0000	0000	
0	0	+.0101	0	+.0003		+.0005		+.0006	0	_	+.0022	0003 +.0027	0004	0004	
		R _X R _Y	+.0101			+.0792		+.0815	-	11.0010	1.0022	1.0027	+.0029	+.0030	
	1.0	0003	0	0000	000ı	0001	0001	0001	0	0	0	0	0	0	
3%	0.8	+.0003	0	0001	000 ı	000ı	0002	0002	0	+.0000		+.0000		+.0000	
	0.6	+.0007	0	0001	0002	0002	0002	0003	0	+.0000	0000	0000		0000	
- "	0.4	+.0023	0	0002	0003	0003	0004	0004	0	0000	0001	0001	0002	0002	
%	0.2	+.0149	0	0004	0005	0005	0004	0004	0	0002	0004	0006		0007	
1 0	<u> </u>	+.0108	0		+.0005		+.0006	+.0006	٥	+.0014	+.0023	+.0028	+.0031	+.0032	
·	1	R _X R _y	+.0108		+.0770		+.0823	+.0826							
	0.8	+.0000	0	0001	0001	0002	0002	0002	0	0	0	0	0	0	
0.6 +.0010 000010002000300030003 0000000010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001000100010001 -														0000	
11 0.4 +.0024 00002000300030003 00001000100010001														0001	
0.2 +.0148 00004000400030003 00003000500060007														0003	
0.2 +.0148 00004000400030003 000030005000600070 0 +.0140 0 +.0004 +.0006 +.0007 +.0007 0 +.0018 +.0028 +.0033 +.0035 +.0														0007	
R _g R _y +.0140 +.0710 +.0798 +.0824 +.0832 +.0833															
R _E R _Y +.0140 +.0710 +.0798 +.0824 +.0832 +.0833															
	R ₂ R _y +.0140 +.0710 +.0798 +.0824 +.0832 +.0833 1.0 +.0012 00001000200030003 0 0 0 0 0 0														
0.8 +.0006 00001000200030003 00000000000010001000															
0.6 +.0010 00001000200030003 0000000010001000200 0.4 +.0022 00002000300030002 000010002000300030003															
% %	0.2	+.0153	0	0004	0003	0002	0002	0002	0			- 0005		0005	
0	0	+.0202	0	+.0005	+.0007	+.0008	+.0008	+.0008	0				+.0040		
\vdash					+.0822	+.0833	+.0836	+.0836							
1 1	1.0	+.0021	0		0002			0003	0	0	0	0	0	0	
-	0.8	+.0005	0					0002	0	0000		0001	0001	1000.	
"	0.6	+.0009	0					0002	0					0002	
8	0.4	+.0166			0002 - 0002 -	0002		0002	0					0003	
0	0.2	+.0252		+.0006+		0002 - +.0008 +		0001 +.0009	0				0004 -		
1 1			$\overline{}$				-	0836	0	+.0028	+.0038 H	1.0041	+.0042	0043	
 	1.0	+.0028	0			0002		0001	0	0	0	0 1	<u> </u>	 _	
3/2	0.8	+.0004	Ö			0002	0001	0001					0001 -	0001	
ا س ا	0.6	+.0007	0	0002		0001			0	0001 -			0001 -		
" [0.4	+.0014	0	0002 -			-	0001	0		0002			0001	
%	0.2	+.0204	0	0003 -	0001	0001	0000 -	0000						.0002	
	0	+.0327		+ 0007 1		+.0009 +	0009	0009						0045	
igsquare		Ry	+ 0327	+ 0810 +	0834	1.0835	+.0835 +	.0835							
Free Hil	ged		·	ż			nt = (Go on = (Go				M:	R, R,	D My	→ X	
· /71/11/17	,,,,,,,	//		->-	P						POSITIV	E SIGN	CONVE	NTION	

FIGURE 24.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load VII, 1/6 uniformly varying load.

]		1	M _x					N			
<u></u>	y/b	R _X X/O	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	1.0	+5.9610	0		+ 0571				0				+1.0000	
<u>~</u>	0.8	-2.8341	0		+.0564				0	+	+		+.0679	
	0.4	0022	0		+.0124	+		-	0	0004	0008	0010	0012	0012
	0.2	+.0016	ő		+.0003				0	0004	- 0008	0011	0013	0014
% 9%	0	+.0030	0	0000	+			+	1	- 0001	0002	0002	0002	0003
		R _X R _Y	+.0030	0011	0020	+	0033	+	<u> </u>	1				
	1.0	+10.3797	0	+.0311		1188			0	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
\\\\7	0.8	-4.3042	0		+.0631		4		0	+.0917	+.1644	+.2161	+.2470	+.2572
"	0.6	7213 1390	0		+.0382				0			 	+.0423	
ما	0.4	+.0220	0		+.0046				0	0014		+.0018 0036	+.0022	
9/p	0	+.0956	0	0010			 	1	0	0051		0134	0158	0166
		R _X R _Y	+.0956	0413	+		+		<u> </u>	1				70.00
	1.0	+13.4266	0	0276	1306	1774	1976	2033	0	+1.0000	+1.0000	+1.0000	+1.0000	+1.00.00
3/8	0.8	-4.9000		+.0421		·	+	+.0044	0	+.1615	+.2810	+.3607	+.4058	+.4204
1	0.6	-1.1940	0	+	+.0409	+		·	0	1	+.0839		+.1363	
"_	0.4	3728	0	+.0134	+	 		+.0383	0	+			+.0368	
9%	0.2	+.0278	0	+.0038 0031	+.0071				0		0022	0029	0032	0033
	 -	R. R.	+.1943	1076		-			<u> </u>	0155	0294	0404	0474	0499
	1.0	+15.7047	0	0725		 			0	+1.0000	+1.0000	+1.0000	+1.0000	+1.0000
~	0.8	-5.1848	0	+.0310	+.0035		+	+	0				+.5207	
1/2	0.6	- I. 4580	0	+.0221	+.0284	+.0257	+.0211	+ 0192	0	+.0786	+.1492	+.2033	+.2367	+.2479
п	0.4	5749	0		+.0215			+.0274	0	+.0294	+.0567	+.0790	+.0935	+.0985
0/p	0.2	0597	0	+	+.0069			+.0096	0	+	+.0096		+.0166	
"	0	+.1393 Rx Ry	0 +.1393	0036	0067 2596	0090	0105	0110	0	0178	0334	0452	0525	0550
-	1.0	+19.0782	0	1285				4200 2451	0	1.0000	+1,0000	+1 0000	+1.0000	+1 0000
_4	0.8	-5.5535	<u> </u>	+.0089				1181	0	-	-		+.6621	
3/4	0.6	-1.7231	0		+.0054			0337	0	+			+.4059	$\overline{}$
11	0.4	9245	0	+.0142	+.0173	+.0136	+.0090	+.0071	0	+.0775	+.1480	+.2027	+.2368	+.2483
٥/p	0.2	4547	0	+.0090	+.0147	+.0173	+.0182	+.0184	0	+.0410	+.0794	+.1109	+.1315	+.1386
0	0	1466	0		+.0065		+.0128	+ 0138	0	+.0150	+.0323	+.0503	+.0640	+.0691
<u> </u>		Rz Ry	1466	1354	2227	_	2627	2615						
	0.8	+21.4049 -5.9317	0	1478 0013	2002 0588	2064 0909	2054 1048	2046 1086	0				+1.0000	
-	0.6	-1.8811	0			0159	0279	0317	0				+.5414	
"_	0.4	-1.2060	0			+.0241			0		+.2537		+ 3922	
۵/ _b	0.2	8257	0	+.0195	+.0326	+.0407	+.0455	+.0471	0		+.1832	+	+.2994	+.3150
	0	3387	0	+.0138	+.0285	+.0420	+.0516	+.0550	0	+.0690	+.1423	+.2101	+ . 2580	+.2751
		R _X R _Y	3387	1330	~.1655	1250	0735	0517						
۱ ا	1.0	+24.0577	0	1416	1333	0989	0729	0635					+1.0000	
3/2	0.8	-6.7445 -1.9745	0	0047	+.0275		0139	0083	_ 0				+.8580 - +.7370 -	
"	0.4	-1.4705	0		+.0577				0		$\overline{}$		+ .6479	
g/b	0.2	-1.1741	0.		$\overline{}$	+.0915					+ 3789		+.5976	
0	0	4225	0		+.0692					+			+.5917	6242
		Ry	4225	1575	1016	+.0130	+.0925	+.1189]
<a>	0-		'		Y M		-,					Ť		*
Free	nged		•					oefficient oefficient			*	R. R.	D D	→ x
annn i	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	<u>+</u>	x		×						w POSITIV	VE SIGN	CONVE	NTION

FIGURE 25.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coefficients, Load VIII, moment at free edge.

					N	l'x	·		1			A _V	*****		
1	У/ь	R _X X/o	0	0.2	0.4	0.6	0.8	1.0		1 0 0		, 	1 0 0	I	
<u> </u>	1.0	+1.2357	0					1.0	0	0.2	0.4	0.6	0.8	1.0	
1	0.8	+.0124	0	0203 0033			0514	0534	0	0	0	0	0	0	
>€	0.6	0039	Ö	0005		0084 0014	0099	0103	0		+.0021		+.0034	+ -	
l n	0.4	0012	0	0001	0002		0017	0017	0				+.0008		
	0.2	0002	 	0000	<u>+</u>	0000	0003	0003	0	+ .0001	+.0001		+.0002		
%	0	0002	0	+.0000	1	+.0000	+.0000	+.0000	0	+.0000	+.0000		+.0000		
	<u> </u>	R _x R _y	0002	+.0001	+.0001	+.0001	+.0002	+.0002	├	1+.0000	+.0000	T.0000	+.0000	[+ .0000	
	1.0	+2.3808	0	0534		1127	1257	1300	0	0	0	0	0	0	
	0.8	+.0825	0	0186	 	0465	0538	0563	Ö		+.0091		+.0140		
74	0.6	0040	0	0069	†——	0178	- 0208	0219	0	 	+.0054		+ 0086		
0	0.4	0117	0	0024	0045	0062	0073	0077	0		+.0025			+.0042	
9%	0.2	0149	0	0006	0011	0015	0017	0018	0		+.0013		+.0021		
°`	0	- 0250	0	+.0002	+.0005	+.0006	+.0008	+ 0008	0	+.0012	+.0023	+.0032	+.0038		
L		R _X R _Y	0250	+.0089	+.0168	+.0228	+.0273	+.0284							
	1.0	+3.4577	0	0873	1422	1761	1947	2007	0	0	0	0	. 0	0	
3/8	0.8	+.1637	0	0401	0730	0966	1107	- 1154	0	+.0087	+.0153	+.0198	+.0223	+.0231	
1	0.6	+.0175	0	0192	0361	0491	0572	0599	0	+.0069	+.0129	+.0174	+.0202	+.0212	
"	0.4	0344	0	0082	0156	0214	0251	0263	0	+.0048	+.0092	+.0126	+.0147	+.0155	
%	0.2	0967	0	0018	0034	0046	0054	0057	0	+.0047	+.0090	+.0124	+.0145	+.0153	
•	0	1369	0	+.0019	+.0037	+.0051	+.0060	+.0063	0	+.0097	+.0185	+.0254	+.0299	+.0314	
		Rx Ry	1369			+.1295	+.1520	+.1599							
1	1.0	+4.5104	0	1183		2294	2519	2590	0	0	0	0	0	0	
7	0.8 +.2138 006141094142316121674 0 +.0117 +.0199 +.0248 +.0274 + 0.6 +.0222 003220596080009240965 0 +.0115 +.0212 +.0282 +.0323 + 11 0.40945 001410265036004190439 0 +.0113 +.0213 +.0291 +.0339 +														
1	11 0.40945 001410265036004190439 0 +.0113 +.0213 +.0291 +.03														
1 1	0.22623 000180033004500520054 0 +.0152 +.0288 +.0396 +.0464														
%	0.22623 000180033004500520054 0 +.0152 +.0288 +.0396 +.0464 +. 02967 0 +.0059 +.0112 +.0154 +.0181 +.0190 0 +.0294 +.0560 +.0769 +.0904 +.														
"	02967 0 +.0059 +.0112 +.0154 +.0181 +.0190 0 +.0294 +.0560 +.0769 +.0904 +														
	R ₈ R ₇ 2967 +.1156 +.2193 +.3010 +.3530 +.3709														
ļ	R ₈ R _y 2967 +.1156 +.2193 +.3010 +.3530 +.3709														
%															
1 1	0.60730 004830853109312231264 0 +.0259 +.0464 +.0605 +.0687 +.0														
1 1	0.60730 004830853109312231264 0 +.0259 +.0464 +.0605 +.0687 +.07 0.43103 001850332042904810497 0 +.0353 +.0661 +.0894 +.1039 +.10														
8	0	5452	0	+.0181				+.0575	0				 †	+ 2876	
1 1		R _x Ry		+.2510		+.6371	+.7396			7.0000		*,2042			
	1.0	+7.7716	0	1796	2494	2779	2885	2911	0	0	0	0 1	0 [-	
	0.8	+.1043	0	1042	1602	1855	1952	1976	0	_	+.0459		+.0609		
l l	0.6	2131	0	0532	0859	1012	1068	1081	0	+.0468	+.0830	+.1080	+.1227	+.1276	
"_	0.4	5179	0	0161	0249	0271	0264	0258	0	+.0686	+.1272	+.1711	+.1981	+.2072	
9/p	0.2	9412	0	+.0120	+.0240	+.0348	+.0421	+.0447	0	+.1020	+.1917	+.2604	+.3032	+.3178	
	0	6348	0	+.0314	+.0588	+.0796	+.0923	+.0966	0	+.1569	+.2940	+.3978	+.4617	+.4831	
		RX	6348	+.3528	+.6445	+.8488	+.9658	+1.0036							
. I	51.0	+9.0313	. 0	1917	-		1933	1866	0	0	0	0]	0	0	
2%	0.8	0267	0		1390				0	+.0501	+.0796	+.0992	+.1112	+.1152	
	0.6	3777	0		0645				0				+.2305		
ا يٰ ا	0.4	7215	0		+.0009				.0				+.3658		
😽	0.2	-1.1699	0		+.0562				0				+.5265		
	0	5965	0	+.0543		+.1280			0	+.2714	+.4904	+.6400	+.7243	+.7513	
$ldsymbol{\sqcup}$		Ry	5965	+.5000	+.8389	+1.0190	+1.0977	+1.1190							
Free H	inged	,,,,,		F				pefficient pefficient			M :	R. Ry	D My	—→ X	
,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	mm	,,,, !	r_x								W POSITI	VE SIG	N CONVI	ENTION	

Figure 26.—Plate fixed along one edge—Hinged along two opposite edges, moment and reaction coafficients, Load IX, line load at free edge.

0.6 + .1253 + .0078 + .0050 + .0028 + .0012 + .0003							Mx			T		N	A _y		
1.0		y/b	Rx X/o	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
0.8		1.0	+.1216	+ .0077	+ .0049	+.0026	+.0012	+ .000	3 0	0	0	0	0	0	0
0.6	8	0.8	+.1267	+ .0079	+.0050	+ .0028	+ .0013	+ .0003	3 0	+ .0016	+.0010	+ .0006	+ .0003	+.0001	+.0000
0.2				ł		+		+ .000	3 0	+.0016	+ .0010	+ .0006	+.0002	+.0001	0000
0004	1		+				+			+ .0015	+ .0010	+ .0005	+ .0002	0000	0001
1.0	\ <u>^</u>				 		+ —	+	+	_	+			0003	0004
1.0 +2285 +0297 +0192 +0108 +0004 +0011 0 0 0 0 0 0 0 0 0	"		-			+	-	+	+	 	+.0007	+ .0023	+.0043	+.0064	+.0083
0.8		1 1 0				+	+		 	 					
0.6 +.2539 +.030			+			+	+	-		+	+		·		
11	74	<u> </u>					+		ļ	·t	+	h			
0.2 + 1.832 + 0.172 + .0092 + 0.040 + .00100003	1 11	0.4	 	-		+		+	+					 	
1.0		0.2	 			-	+	-	+	t				 	
1.0	0	0	0866	0	+.0006	+.0017		 	+	t	t				
0.8 +.4129 +.0664 +.0399 +.0206 +.0078 +.0010 0 +.0133 +.0076 +.0031000200250037 0.6 +.3798 +.0604 +.0356 +.0176 +.0060 +.0002 0 +.0121 +.0067 +.0020001800450060 0.4 +.3439 +.0479 +.0265 +.0120 +.00320007 0 +.0096 +.00450000003600620078 0.2 +.1762 +.0232 +.0119 +.0051 +.0016 +.0001 0 +.0046 +.0020 +.0003000700110012 0.00155 0 +.0011 +.0034 +.0061 +.0092 0 0 +.0046 +.0020 +.0033000700110012 0.8 +.5570 +.1052 +.0059 +.0051 +.0013 +.0056 +.0099 0 0 +.0055 +.0168 +.0307 +.0458 +.0589 0.8 +.5570 +.1052 +.0059 +.0334 +.00610017 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Ĺ		R _X R _y	0866	0058	+ .0998	+ .2009	+.3764	+ .5716	i –				.0200	1.0004
0.8		1.0	+.3267	+.0642	+.0407	+.0218	+.0085	+.0012		0	0	0	0	0	0
0.4 +.3439 +.0479 +.0265 +.0120 +.00320007		0.8	+.4129	+.0664	+.0399	+.0206	+.0078	+.0010	0	+.0133	+.0076	+.0031	0002	0025	
0.2 +.1762 +.0232 +.0119 +.0051 +.0016 +.0001 0 +.0046 +.0020 +.0003000700110012 00155 0 +.0011 +.0034 +.0061 +.0092 0 0 +.0055 +.0168 +.0307 +.0458 +.0589 R ₁ R ₂ 01550080 +.1135 +.2213 +.4296 +.6709 1.0 +.4597 +.1074 +.0638 +.0304 +.00650017 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ω,		_				+.0060	+.0002	0	+.0121	+.0067	+.0020	0018	0045	0060
0 -0.0155	11					 			0	+.0096	+.0045	0000	0036	0062	0078
R _x R _y 0155 0080 +.1135 +.2213 +.4296 +.6709 0016 0017 0 0 0 0 0 0 0 0 0	Q a					·				+.0046	+.0020	+.0003	0007	1100	0012
1.0	0,	-								0	+.0055	+.0168	+ .0307	+ .0458	+ .0589
0.8 +.5570 +.1052 +.0592 +.0275 +.00790010 0 +.0210 +.0108 +.0028003200760105															
0.6								 -							
0.4	1/2											+			
0.2 +.1422 +.0261 +.0134 +.0062 +.0031 +.0019 0 +.0052 +.0033 +.0035 +.0051 +.0071 +.0088 00401 0 +.0019 +.0059 +.0108 +.0163 0 0 +.0096 +.0296 +.0296 +.0541 +.0813 +.1051 Rx Ry0401 +.0011 +.1576 +.3024 +.5696 +.8739 1.0 +.8290 +.1977 +.0952 +.029800590162 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0															
00401 0 +.0019 +.0059 +.0108 +.0163 0 0 +.0096 +.0296 +.0541 +.0813 +.1051 R_X R_Y 0401 +.0011 +.1576 +.3024 +.5696 +.8739		0.2	+.1422					·					+		
Rx Ry - 0.401 +.0011 +.1576 +.3024 +.5696 +.8739	_	0	0401	0	+.0019				i						
0.8 +.7827 +.1726 +.0817 +.026100320110 0 +.0345 +.01440001016601860257 0.6 +.5677 +.1318 +.0621 +.019400230079 0 +.0264 +.01020034013902180279 0.4 +.3914 +.0826 +.0377 +.0120 +.00010026 0 +.0165 +.00610005004200630077 0.2 +.0741 +.0282 +.0144 +.0077 +.0072 +.0081 0 +.0056 +.0084 +.0176 +.0296 +.0411 +.0501 00698 0 +.0041 +.0125 +.0221 +.0326 0 0 +.0026 +.0623 +.1104 +.1630 +.2076 R _x R _y 0698 +.0333 +.2595 +.4574 +.7928 +1.1288 1.0 +1.1828 +.2949 +.1046 +.014602680324 0 0 0 0 0 0 0 0 0 0.8 +.9335 +.2421 +.0873 +.012901990227 0 +.0484 +.01590023014102270324 0.6 +.5948 +.1724 +.0643 +.009701320141 0 +.0345 +.01190032013201990268 0.4 +.3699 +.1033 +.0384 +.006900320023 0 +.0207 +.0090 +.0069 +.0097 +.0129 +.0146 0.2 +.0548 +.0362 +.0152 +.0090 +.0119 +.0159 0 +.0072 +.0152 +.0384 +.0643 +.0873 +.1046 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0072 +.0152 +.0384 +.0643 +.0873 +.1046 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949			R _X R _Y	0401	+.0011	+.1576	+. 3024	+.5696	+.8739						
0.6 +.5677 +.1318 +.0621 +.019400230079 0 +.0264 +.01020034013902180279 0.4 +.3914 +.0826 +.0377 +.0120 +.00010026 0 +.0165 +.00610005004200630077 0.2 +.0741 +.0282 +.0144 +.0077 +.0072 +.0081 0 +.0056 +.0084 +.0176 +.0296 +.0411 +.0501 00698 0 +.0041 +.0125 +.0221 +.0326 0 0 +.0206 +.0623 +.1104 +.1630 +.2076 R _X R _Y 0698 +.0333 +.2595 +.4574 +.7928 +1.1288 1.0 +1.1828 +.2949 +.1046 +.014602680324 0 0 0 0 0 0 0 0 0 0.8 +.9335 +.2421 +.0873 +.012901990227 0 +.0484 +.01590023014102270324 0.6 +.5948 +.1724 +.0643 +.009701320141 0 +.0345 +.01190032013201990268 0.4 +.3699 +.1033 +.0384 +.006900320023 0 +.0207 +.0090 +.0069 +.0097 +.0129 +.0146 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0072 +.0152 +.0384 +.0643 +.0873 +.1046 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949		1.0	+ . 8290	1977	+.0952	+.0298	0059	0162	0	0	0	0	0	0	0
11	, 4	0.8	+.7827	+.1726	+.0817	+.0261	0032	0110	0	+.0345	+.0144	0001	0106		
0.2 +.0741 +.0282 +.0144 +.0077 +.0072 +.0081 0 +.0056 +.0084 +.0176 +.0296 +.0411 +.0501 00698 0 +.0041 +.0125 +.0221 +.0326 0 0 +.0206 +.0623 +.1104 +.1630 +.2076	3/	0.6	+.5677	+.1318	+.0621	+.0194	0023	0079	0	+.0264	+.0102	0034	0139	0218	0279
00698 0 +.0041 +.0125 +.0221 +.0326 0 0 +.0206 +.0623 +.1104 +.1630 +.2076 R _X R _Y 0698 +.0333 +.2595 +.4574 +.7928 +1.1288 1.0 +1.1828 +.2949 +.1046 +.014602680324 0 0 0 0 0 0 0 0 0 0 0.8 +.9335 +.2421 +.0873 +.012901990227 0 +.0484 +.01590023014102270324 0.6 +.5948 +.1724 +.0643 +.009701320141 0 +.0345 +.01190032013201990268 0.4 +.3699 +.1033 +.0384 +.006900320023 0 +.0207 +.0090 +.069 +.0097 +.0129 +.0146 0.2 +.0548 +.0362 +.0152 +.0090 +.0119 +.0159 0 +.0072 +.0152 +.0384 +.0643 +.0873 +.1045 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949	11					+.0120	+ .0001	0026	0	+.0165	+.0061	0005	0042	- 0063	0077
1.0				+					0	+.0056	+.0084	+ .0176	+ .0296	+.0411	+.0501
1.0 +1.1828 +.2949 +.1046 +.014602680324 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	ָ ס		 1							0	+.0206	+ .0623	+ .1104	+.1630	+.2076
0.8 +.9335 +.2421 +.0873 +.012901990227 0 +.0484 +.01590023014102270324 0.6 +.5948 +.1724 +.0643 +.009701320141 0 +.0345 +.01190032013201990268 0.4 +.3699 +.1033 +.0384 +.006900320023 0 +.0207 +.0090 +.0069 +.0097 +.0129 +.0146 0.2 +.0548 +.0362 +.0152 +.0090 +.0119 +.0159 0 +.0072 +.0152 +.0384 +.0643 +.0873 +.1046 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949				+								- г			
0.6 + .5948 + .1724 + .0643 + .009701320141 0 + .0345 + .01190032013201990268 0.4 + .3699 + .1033 + .0384 + .006900320023 0 + .0207 + .0207 + .0090 + .0069 + .0097 + .0129 + .0146 0.2 + .0548 + .0362 + .0152 + .0090 + .0119 + .0159 0 + .0072 + .0152 + .0384 + .0643 + .0873 + .1046 00887 0 + .0072 + .0207 + .0345 + .0484 0 0 + .0362 + .1033 + .1724 + .2421 + .2949	 									+					
0.4 + .3699 + .1033 + .0384 + .006900320023 0 + .0207 + .0090 + .0069 + .0097 + .0129 + .0146 0.2 + .0548 + .0362 + .0152 + .0090 + .0119 + .0159 0 + .0072 + .0152 + .0384 + .0643 + .0873 + .1046 00887 0 + .0072 + .0207 + .0345 + .0484 0 0 + .0362 + .1033 + .1724 + .2421 + .2949	└ ~ ├														
0.2 +.0548 +.0362 +.0152 +.0090 +.0119 +.0159 0 +.0072 +.0152 +.0384 +.0643 +.0643 +.0873 +.1046 00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949 Ry0887 +.0548 +.3699 +.5948 +.9335 +1.1828			-											+	
00887 0 +.0072 +.0207 +.0345 +.0484 0 0 +.0362 +.1033 +.1724 +.2421 +.2949 Ry0887 +.0548 +.3699 +.5948 +.9335 +1.1828	امح														
Ry0887 + .0548 + .3699 + .5948 + .9335 +1.1828	°				+						+	\longrightarrow			
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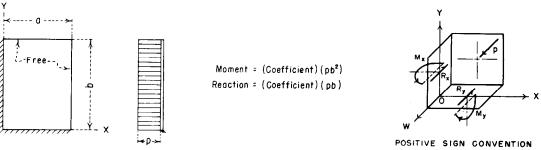


Figure 27.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load I, uniform load.

					٨	A x					N	l _y			
	y/b	R _X X/o	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0	
	1.0	0014	+.0002	+.0002	+.0002	+.0001	+.0001	0	0	0	0	0	0	0	
8/	0.8	+.0075	+.0011	+.0008	+.0006	+.0004	+.0002	0	+ 0002	+.0002	+ .0002	+ 0002	+.0003	+.0003	
-`	0.6	+.0970	+.0056	+.0035	+.0019	+.0008	+.0001	0	+.0011	+.0007	+ .0003	+.0000	0001	0002	
j o .	0.4	+.1258	+.0074	+.0046	+.0025	+.0010	+.0002	0	+.0015	+ .0009	+.0004	+.0001	0002	0003	
% %	0.2	+.1209	+.0067	+.0040		+ .0008	+.0001	0		+ 0008	+ .0003	0001		0005	
°	0 .	+.0046	0	· · · · · · · · · · · · · · · · · · ·	+ .0005		+ .0013	0	0	+.0007	+ .0023	+.0043	[+.0064	+.0083	
—	1.0	0187	+.0046	+.0031	+.0505	+ 0020	+.2023	T	0.	0	0	0			
	0.8	+.0315	ł	1	+.0041	+ .0023	+ .0009	0	+ 00 6		+.0014	_	+.0016	+ .0018	
74	0.6	+ 1899	+.0195			+ .0019	+.0000	0		+.0021	+ 6005		0017	0022	
,,	0.4	+ 2492	+ 0238		+.0060	+.0015	0005	0	· .	+.0023	+.0001	ł -	0031	0040	
	0.2	+.1869	+.0165		+.0033	+.0005	0006	0		+.0013	0005		0030	0037	
۵%	0	0822	0	+.0006	+.0017	+.0031	+ 0045	ō	ا أ	+ 0028			+.0226	• •	
		R _X R _Y	0822	0012	+.1050	+.2030	+.3681	+.5432		<u></u>			•		
	1.0	0462	+.0102	+.0106	+.0085	+.0055	+.0026	0	0	0	0	0	0	0	
8	0.8	+.0819	+.0213	+.0149	+.0091	+.0046	+.0015	0	+.0043	+ 0033	+.0025	+.0019	+ 0016	+.0017	
3,	-0.6	+ .2733	+.0361		+.0083	+.0017	0010	0	+ .0072	+ 0030	0008	0039	0061	- 0076	
"	0.4	+.3352	+.0384		+ .0063	- 0003	0022	0	 	+.0024	0025		0096	0118	
9	0.2	+.1928	+.0212		+.0027	0003	- 0010	0	+			0030		- 0049	
0	00069 0 +.0011 +.0031 +.0055 +.0079 0 0 +.0055 +.0157 +.0274 +.0395 +.04 R _x R _y 0069 +.0125 +.1333 +.2285 +.3963 +.5629 1, 00487 +.0223 +.0201 +.0137 +.0074 +.0028 0 0 0 0 0 0 0 0 0														
\vdash	1. 00487 + .0223 + .0201 + .0137 + .0074 + .0028 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
	1. b0487 +.0223 +.0201 +.0137 +.0074 +.0028 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
/2	·		· · ·											- 0154	
١.,	0.6 + .3336 + .0502 + .0244 + .0086 + .00010026 0 + .0100 + .002900360089012701														
مّ ا	0.6 + .3336 + .0502 + .0244 + .0086 + .00010026 0 + .0100 + .0029003600890127015 0.4 + .3772 + .0476 + .0198 + .004600250038 0 + .0095 + .0013006101190161019														
0	0	0250	0	+.0019	+.0053	+.0089	+.0125	0	0	+.0096	+.0263	+.0443	+.0625	+ 0775	
		R _X	0250	+.0438	+ .1939	+.3071	+.4893	+ .6544							
•	1.0	+.0368	+.0524	+.0341	+.0153	+.0028	0022	0	0	0	0	0	0	0	
3/4	0.8	+.2262	+.0612	+.0319	+.0118	+.0005	0029	0	+.0122	+.0059	+ .0004	0042	0078	0106	
ω,	0.6	+.3844			+ 0046	0047	~.0059	0	 	+.0006	0102	0182	0238	0279	
"	0.4	+.3913	+.0541	+.0165	0002	- 0057	0052	0	+.0108	0019	- 0117	0180	0217	0244	
q/ ₀	0.2	+.1509	+ 0229		+.0006	+.0006	+.0021	0	+ .0046	+.0018	+.0034		+.0119	+.0154	
		R _X	-			+.4261	+.6038	+.7410		1.0200	1.0434	7.0773	1.1043	7.1202	
	1.0	+.1522	+ 0776		+.0086	0061	0088	0	0	0	0	0	0	0	
	0.8	+.2785	+.0772	+.0318	+.0051	0067	0076	0	+.0154	+.0058	0018	0075	0117	0156	
-	0.6	+.3929	+.0725	+.0216	0016	0096	0085	0	+.0145	0022	0145	0224	0274	0316	
ا ا	0.4	+.3794	+.0542	+.0113	0042	0074	0051	0		0046			0182	0193	
۵/p	0.2	+.1311	+.0216	+.0042	+.0007	+.0027	+.0054	0	+.0043	+.0036			+.0292	+.0360	
	0	0499	0	+.0065	+.0145	+.0213	+.0275	0	0	+.0323	+.0725	+.1064	+.1375	+.1605	
L _		, EX	0499	+ 1916	+.3934	+.5067	+.6597	+ . 7476	L						
	Free	Q	X Y	▼ p ->-			= (Coef				W POSITIV	R _x R _y	M, conve	X	

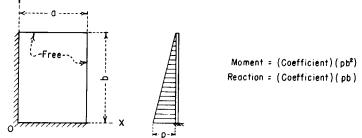
Figure 28.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load II, 2/3 uniform load.

RESULTS

1.0 -0.001 +.0000 +.0000 +.0000 +.0000 +.0000 0 0 0 0 0 0 0 0							M _x		•	1		1	M _y			
1.0		У/ь	R _X X/a	0	0.2	0.4	0.6	0.8	1.0	0	0.2	т——	-	0.8	1.0	
0.60001 +.0002 .0002 .0002 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0001 .0		1.0	0001	+.0000	+ .0000	+.0000	+ .0000	+.0000	0	0	0	0	 	0	0	
0.8		0.8	0002	+.0000	+.0000	+.0000	+.0000	+.0000	0	+.0000	+.0000	+.0000		<u> </u>	+.0000	
0.4 +.0288 +.0021 +.0014 +.0008 +.0004 +.0001 0 +.0011 +.0008 +.0001 +.0001 +.0001	-	0.6	0001	+.0002	+ .0002	+.0002	+.0001	+.0001	0	+.0000	+.0001	+.0001	├		+.0001	
0.2 + 1.089 + .0097 + .0032 + .0013 + .00040001 0 + .0011 + .0006 + .000100040007 + .0005	- 11	0.4	+.0288	+.0021	+.0014	+.0008	+.0004	+.0001	0	+.0004	+.0003	+.0002	+.0001	+.0001	+.0001	
0 + .0441 0 + .0001 + .0003 + .0006 + .0009 0 0 + .0006 + .0017 + .0030 + .0045	ما	0.2	+.1089	+.0057	+.0032	+.0014	+.0004	0001	0	+.0011	+.0006	+.0001	0004	0007	0009	
1.0 -0.034 +.0.002 +.0.003 +.0.003 +.0.003 +.0.0002 0 0 0 0 0 0 0 0 0	6	0	+.0441	0	+.0001	+.0003	+.0006	+.0009	0	0	+.0006	+.0017	+.0030	+.0045	+.0057	
0.80003 +.0007 +.0007 +.0006 +.0004 +.0002			R _X R _y	+.0441	+.0021	+.0412	+.0763	+.1395	+.2216							
0.6 +.0049 +.0023 +.0014 +.0013 +.0008 +.0003		1.0	0034	+.0002	+.0003	+ .0003	+ .0003	+.0002	0	0	0	0	0	0	0	
10	4	0.8	0003	+.0007	+.0007	+.0006	+.0004	+.0002	0	+.0001	+.0002	+ .0002	+ .0003	+.0004	+.0004	
0.2 + .1620 + .0113 + .0046 + .000900080011 0 + .0023 + .0003001500300041 0 + .0298 0 + .0004 + .0012 + .0019 + .0027 0 0 0 + .0021 + .0058 + .0096 + .0135 R, R, Y + .0298 + .0247 + .0989 + .1533 + .2384 + .3108 000120 + .0010 + .0014 + .0013 + .0010 + .0005 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	-	0.6	+.0049	+.0023	+.0019	+.0013	+.0008	+.0003	0	+.0005	+.0005	+.0005	+.0006	+.0006	+.0007	
0 + .0298	п	0.4	+.0655	+.0071	+.0041	+.0019	+.0005	0001	0	+.0014	+ .0008	+.0002	0002	0006	0009	
1.0	م ا	0.2	+.1620	+.0113	+.0046	+.0009	0008	0011	0	+.0023	+ .0003	0015	0030	0041	0050	
1.0 0120 +.0010 +.0014 +.0013 +.0010 +.0005 0 0 0 0 0 0 0 0 0	0	0		0	+.0004	+.0012	+ .0019	+.0027	0	0	+.0021	+.0058	+.0096	+ .0135	+.0165	
0.6 +.0047 +.0028 +.0023 +.0017 +.0010 +.0005 0 +.0006 +.0005 +.0006 +.0006 +.0007 +.0006		<u> </u>	R _x R _y	+.0298	+.0247	+.0989	+.1533	+ .2384	+.3108							
0.6 +.0178 +.0055 +.0038 +.0022 +.0010 +.0002 0 +.0011 +.0010 +.0008 +.0007 +.0008	1	1.0	-					+.0006	0	0	0	0	0	0	0	
1	&								_					+ .0007	+.0009	
0.2 + .1803 + .0134 + .0037000600200017	w,	0.6	+.0178	+.0055	+.0038	+.0022	+.0010	+ .0002	0	+.0011	+.0010	+ .0008	+ .0007	+.0006	+ .0005	
0 + .0163	11	0.4	+.0923					0008		+.0022	+.0008	0006	0018	0028	0035	
1.0	0 +.0163 0 +.0008 +.0019 +.0030 +.0040 0 0 +.0039 +.0096 +.0151 +.0201 +.02														0076	
1.0 0177 +.0025 +.0029 +.0024 +.0015 +.0007 0 0 0 0 0 0 0 0 0															+.0238	
0.8 + .0130 + .0052 + .0038 + .0024 + .0013 + .0004 0 + .0010 + .0008 + .0007 + .0006 + .0006																
0.6 + .0299 + .0084 + .0050 + .0023 + .00060001		1.00177 +.0025 +.0029 +.0024 +.0015 +.0007 0 0 0 0 0 0 0 0														
0.4 +.1045 +.0133 +.0051 +.000600120014 0 +.0027 +.0004001800370050 0.2 +.1774 +.0131 +.0020001700240017 0 +.00260017004700640073 0 +.0086 0 +.0013 +.0029 +.0042 +.0054 0 0 +.0064 +.0144 +.0211 +.0268 0 +.0086 0 +.0013 +.0029 +.0042 +.0054 0 0 +.0064 +.0144 +.0211 +.0268 0 +.0086 +.00973 +.1953 +.2459 +.3116 +.3434 00104 +.0067 +.0053 +.0028 +.0009 +.0000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	~		1	-											+.0006	
0.2 +.1774 +.0131 +.0020001700240017 0 +.00260017004700640073 0 +.0086 0 +.0013 +.0029 +.0042 +.0054 0 0 +.0064 +.0144 +.0211 +.0268	-`	⊢—		\vdash				-							0008	
0 +.0086 0 +.0013 +.0029 +.0042 +.0054 0 0 +.0064 +.0144 +.0211 +.0268	•														0060	
1.0	٩		-	_											0080	
1.00104 +.0067 +.0053 +.0028 +.0009 +.0000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	0	-	_								+.0064	T.0144	Ŧ.0211	T .0266	+.0310	
0.8 + .0269 + .0091 + .0054 + .0023 + .00050002		+												0	0	
0.6 + .0411 + .0115 + .0051 + .001200060009															0010	
0.4 + .1086 + .0141 + .0032001200220017	1 %		-				-								0035	
0.2 + .1701 + .01130003002500200011 0 + .00230034005800610056006100560048 0 + .0048 + .1615 + .2512 + .2874 + .3312 + .3489 1.0 + .0052 + .0104 + .0059 + .001800060011 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0															0083	
0 + .0048 0 + .0023 + .0045 + .0060 + .0072 0 0 + .0117 + .0224 + .0299 + .0358024 + .0048 + .1615 + .2512 + .2874 + .3312 + .3489 1 . 0 + .0052 + .0104 + .0059 + .001800060011 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			-										-		0054	
R _X R _Y + .0048 + .1615 + .2512 + .2874 + .3312 + .3489 1. 0 + .0052 + .0104 + .0059 + .001800060011 0 0 0 0 0 0 0 0 0. 8 + .0356 + .0116 + .0053 + .001100080010 0 + .0023 + .0012 + .000200070014 - 0. 6 + .0430 + .0126 + .0041000200160013 0 + .0025 + .0008001200280038 - 0. 4 + .1052 + .0135 + .0013002200250017 0 + .00270021005300690075 - 0. 2 + .1682 + .00940015002300150005 0 + .00190045005500430029 - 0 + .0088 0 + .0033 + .0057 + .0072 + .0083 0 0 + .0166 + .0285 + .0358 + .0414 - R _Y + .0088 + .2052 + .2808 + .3064 + .3372 + .3473	15		-												+.0402	
1. 0	-															
0.8 +.0356 +.0116 +.0053 +.001100080010 0 +.0023 +.0012 +.000200070014 - 0.6 +.0430 +.0126 +.0041000200160013 0 +.0025 +.0008001200280038 - 0.4 +.1052 +.0135 +.0013002200250017 0 +.00270021005300690075 - 0.2 +.1682 +.00940015002300150005 0 +.00190045005500430029 - 0 +.0088 0 +.0033 +.0057 +.0072 +.0083 0 0 +.0166 +.0285 +.0358 +.0414 - Ry +.0088 +.2052 +.2808 +.3064 +.3372 +.3473		1.0		 						0	0	0	0	0	0	
0.6 +.0430 +.0126 +.0041000200160013	1		\vdash					-					0007		0020	
0. 4 +.1052 +.0135 +.0013002200250017 0 +.00270021005300690075002300150005 0 +.0019004500550043002900150088 0 +.0033 +.0057 +.0072 +.0083 0 0 +.0166 +.0285 +.0358 +.04140088 +.2052 +.2808 +.3064 +.3372 +.3473	-		\vdash					-							0046	
0. 2 + .1682 + .00940015002300150005 0 + .0019004500550043002900450088 0 + .0033 + .0057 + .0072 + .0083 0 0 + .0166 + .0285 + .0358 + .04140088 + .2052 + .2808 + .3064 + .3372 + .3473	"		-						0	+.0027	0021		 +		0080	
0 + .0088 0 + .0033 + .0057 + .0072 + .0083 0 0 + .0166 + .0285 + .0358 + .04140285 + .0088 + .2052 + .2808 + .3064 + .3372 + .3473	%														0020	
Ry + .0088 + .2052 + .2808 + .3064 + .3372 + .3473															+.0456	
Moment = (Coefficient) (pb ²) Reaction = (Coefficient) (pb)						-						•	1			
W POSITIVE SIGN CONVE	F	ree	x	4								W SOCIT	№) My	→ X	

FIGURE 29.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load III, 1/3 uniform load.

						Иx					N	1 _y		
	y/b	R _X X/o	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0
	1.0	+.0075	+.0007	+.0006	+.0004	+.0002	+.000	0	0	0	0	0	0	0
_%	0.8	+.0252	+.0017	+.0011	+.0006	+ .0003	+.0001	0	+ .0003	+.0002	+.0001	+:0001	+.0001	
-`	06	+ .0501	+ .0031	+ .0020	+.0011	+ .0005	+.0001	0	+ .0006	+ .0004	+.0002	+.0001	+ .0000	
l n	0.4	+ .0755	+.0046	+ 0029		+ .0007	+.0001	0	+ .0009	+ .0006	+ .0003	+.0001	0000	
9%	0.2	+.0964	+.0052	+.0031	+.0015		+ .0000	0	+.0010	+.0006	+ .0002	0001	0003	0004
0	0	+ .0056	0	+.0001	+ . 0004	+ .0008	+.0011	0	0	+.0007	+ .0021	+.0038	+.0056	+.0072
ļ	ļ <u>.</u>	Rx Ry	+.0056	0008	+.0510	+ .0996	+.1819	+ . 2706	<u>. </u>					
İ	1.0	+.0076	+.0043	+.0035	+.0026	+.0016	+.0007	0	0	0	0	0	0	0
7/	0.8	+ .0557	+.0078	+.0052	+.0031	+.0015	+.0005	0	+.0016	+ .0011	+.0007	+.0005	+.0004	+.0004
	0.6	+.1026	+.0119	+.0074	+.0039	+.0015	+.0003	0_	+.0024	+.0014	+.0007	+.0001	0004	0006
11	0.4	+.1513	+.0150	+.0085	+ 0040	+.0011	0002	0_	+.0030	+.0015	+.0002	0009	0017	0022
g/b	0.2	+.1475	+.0122	+.0060	+.0022	+.0001	0006	0	+.0024	+ .0008	0006	0017	0026	0032
"	0	0598	0	+.0005	+.0014	+.0024	+.0035	0	0	+.0024	+.0069	+.0121	+.0175	+.0221
		R _X R _Y	0598	+.0133	+.1020	+ .1780	+.0015	T -	<u> </u>					
	0.8	+ .0040	+.0115	+.0095	+.0066	0	0	0	0	0	0	0		
3/8	0.6	+ .1553	+.0233	+.0133	+.0066	0	+ .0036		+.0012	+.0004	0000	0002		
1	0.4	+.2050	+ 0246		+.0062	0	+ 0047	+.0024	+.0003	0013	0026	0034		
- 11	0.2	+.1517	+.0152	+.0122	+.0045	0_	+.0049	+.0017	0012	0037	0055	0067		
9/p	0.2	+.0044	0	+.0009	+.0016	0	+.0030	+ 0006	0014	- 0028	0037	0043		
		R X Ry	+.0044	+ .0309	+ 0024	0	0	+ .0044	+ .0119	+ .0202	+.0286	+.0354		
<u> </u>	1.0	+.0166	+.0221	+.0167	+.0100	+.4185	_		1					
	0.8	+ .1402	+.0295	+.0177	+.0090	0	0 +.0059	0	0	0	0	0		
72	0.6	+.1953	+.0334	+.0174	+.0069	+.0032	+.0003 0013	0	+ .0067	+.0032	+.0010	0007	0019	0026
l I	0.4	+ .2311	+.0309	+.0135	+.0037	0011	0021	0	+.0062	+.0012	0010 0032	0041	0064	0079
"	0.2	+.1413		+.0058	+.0010	0005	- 0004	0	-	+.0004	0012	0018	0092	0109
8	0	0079	0	+.0015	+.0039	+.0064	+.0089	0	0		+.0193	+.0318	+.0443	0017 +.0546
		R _X R _y	0079					+ .4827		1.0011		1.0318	1.0443	+.0346
	1.0	+.0974	+.0465			8000.+	0028	0	0	0	0	0	0	0
_	0.8	+.2067	+.0487		+.0084	0002	0027	0		+.0040	0004	0039	0065	0087
3/4	0.6	+.2303	+.0459		+.0046	0023	- 0036	0	+.0092	+.0020	0044	0093	0129	0155
	0.4	+.2383	+.0360	+.0121	+.0010	0030	0029	0	+.0072	0003	0060	0096	0117	- 0132
	0,2	+.1193	+.0160	+.0042	+.0005	+.0006	+.0016	0	+.0032		+.0019	+.0050	+.0083	+.0108
q/p	0	0194	0	+.0029	+ .0070	+.0110	+.0148	0	0		+.0352			+.0896
		Rx	0194	+.1105	+.2399	+.3236	+ . 4489	+.5505	<u>.</u>			1		
	1.0	+.1917	+.0662	+.0291	+.0056	0059	0077	0	0	0	0	0	0	0
	0.8	+.2481	+.0613	+.0243	+.0035	0056	0063	0	+.0123	+.0039	0019	0059	0089	0118
- [0.6	+.2364	+.0518	+.0173	+.0004	0059	0054	۵	+.0104	+.0009	0064	0112	0144	0172
ם ־	0.4	+.2289	+ .0368	+.0092	0015	0041	0028	0	+.0074	0015	0062	0078	0080	0084
٩/٥	0.2	+.1047	+.0150	+.0030	+.0007	+.0021	+.0040	0	+.0030	+.0022	+.0073	+.0148	+.0216	+.0268
	0	0224	0	+.0046	+.0103	+.0152	+.0197	0	0	+.0232	+.0515	+ 0759	+.0987	+.1157
		P.Z	0224	+.1598	+.2991	+.3794	+ . 4909	+.5586						
Y	- (1			-						-		Y		



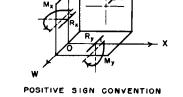


FIGURE 30.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load IV, uniformly varying load.

					N	Λ _x					N	/ _y			
ĺ	У/ в	RX X/O	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0	
	1.0	0003	+.0000	+.0000	+.0000	+.0000	+.0000	0	0	0	0	0	0	0	
, ∞	0.8	+.0001	+.0001	+.0001	+.0001	+.0001	+.0000	0	+.0000	+ 0000	+.0000	+.0001	+.0001	+ 0001	
	0.6	+.0144	+.0011	+.0007	+.0004	+.0002	+.0001	0	+ .0002	+.0002	+ 0001	+.0001	+.0001	+.0001	
ti	0.4	+.0504	+.0030	+.0019	+.0010	+.0004	+.0001	0	+ 0006	+.0004	+ 0002	+.0001	- 0000	0001	
۹	0.2	+.0843	+.0044	+.0026	+.0013	+.0004	+.0000	0	+ .0009	+.0005	+.0002	0001	- 0003	0004	
6	0	+.0063	0	+.0001	+.0004	+.0007	+.0010	0	0	+.0007	+.0020	+.0035	+.0052	+.0066	
		<u> </u>	+.0063	+.0019		+.0964	+. 1713			·	T			,	
	1.0	0057			+.0007	+.0005	+.0003	0 -	0	0		0	0	0	
-74	0.8	+ .0031	+.0017	+.0014	+.0011	+.0007	+.0003	0	+.0003	+.0003		+.0005	+.0006	+.0007	
1	0.6	+.0331	+.0048	ļ	+.0019	+.0009	+.0002	0 -	+.0010	+.0007		+.0004	+ 0003	+.0002	
111	0.4	+ 1010	+.0096 +.0098	+.0053	+.0023	+.0005 0002	0002	0	+ 0019	+.0009	0007	0007 0018	0013	0017	
a/ _b	0.2	0472	0	:	+.0012	+.0021	+.0029	0	0	+.0022		+.0103	+.0146	+.0181	
	├ ॅ	Rx Ry	0472		-	+.1661	+ 2646	+.3544	<u> </u>	1.0022	1.0001	14.0103	1+.0146	7.0181	
\vdash	1.0	~.01 69	+.0022	+.0027	+.0023	+.0016	+.0008	0	0	0	0	0	0	0	
60	0.8	+.0147	+.0053	+.0040	+.0027	+.0015	+.0006	0	+.0011	+.0009	+.0009	+.0008	+.0009		
3/1	0.6	+.0565	+.0099	+.0059	+.0029	+.0010	0000	0	+.0020	+.0012	+.0005	0001	0006	0009	
l u	0.4	+.1351	+.0148	+.0069	+.0020	0004	0011	0	+.0030	+.0008	0012	0029	0042	0051	
٩	0.2	+.1353	+.0117	+.0040	+.0003	0011	0011	0	+.0023	+.0000	0019	0033	0042	0049	
6	0	+.0125	0	+.0008	+.0019	+.0031	+.0043	0	0	+.0038	+.0097	+.0156	+.0213	+.0257	
	R _X R _y +.0125 +.0460 +.1236 +.1740 +.2538 +.3159 1.00220 +.0052 +.0053 +.0039 +.0023 +.0010 0 0 0 0 0 0 0														
	1.00220 +.0052 +.0053 +.0039 +.0023 +.0010 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
_ω	0.8 +.0298 +.0095 +.0065 +.0038 +.0018 +.0005 0 +.0019 +.0014 +.0010 +.0006 +.0004 +.0003														
-															
В	0.6 +.0753 +.0143 +.0076 +.0030 +.00040005 0 +.0029 +.00140001001400250031 0.4 +.1508 +.0178 +.0068 +.001000150017 0 +.0036 +.00020028005200700082														
₀/þ	0.2	+.1313	+.0119	+.0030	0005	0014	0010	0	+.0024	0005	0025	0036	0041	0044	
0	٥	+ .0050	0	+.0013	+.0030		+.0060	0	0	+.0063	+.0148	+.0228	+.0301	+.0358	
L		R _X R _Y	+ .0050	+.0755	-	+.2132		+.3382							
	1.0	0036	+.0130	1	+.0045		0003 0006	0	+.0033	+ .0018	+ .0005	0 0007	0 0016	0023	
3/4	0.8	+.0540	+ 0191		+.0016	0012	0016	0	+.0038	+.0009		0043	0060		
1 1	0.6	+.1561	+.0193		0011	0012	0022		+.0039	0014	0055	0081	0097		
"	0.2	+ 1218	+.0110		- 0012		- 0002	0	+.0022	0013		0017	0006		
٥/p	0	0000	0		+ 0049		+.0088	0	0	+.0118	+.0246		+.0438	+.0507	
	_	R _X R _Y	0000	+.1274	+.2163	+.2612	+.3210	+.3569							
	1.0	+.0261	+.0198			0013	0022	0	0	0	0	0	0	0	
	0.8	+.0687	+.0207	+.0089	+.0017	0016	0019	0	+.0041	+.0018	0001	0016	0028	0039	
-	0.6	+.0943	+.0209	+.0064	0004	0027	0023	0	+.0042	+.0002	0033	0058	0074	0086	
ا ا	0.4	+.1522	+.0188	+.0026	- 0024	0032	0022	0	+.0038	0028	0068	0086	0092	0098	
٩/٥	0.2	+.1168	+.0096				+.0008	0	+.0019	0016	0008	+.0016		+.0059	
!	0	+.0018	0	+.0035	+.0066	+.0088	+.0107	0	0	+.0174	+.0329	+.0440	+.0534	+.0603	
		<u> </u>	+.0018	+.1676	+ .2512	+.2879	+.3348	+.3567							
Y	- 0	, q	× ×				(Coeffic				Mx (Y R. R.	ДР 	— → X	
0.,,,,		-		p 🖳						PC	SITIVE	SIGN C	CONVEN	TION	

Figure 31.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load V, 2/3 uniformly varying load.

	Ĭ				1	M _x			7		٨	vi y		·
1	У/Ь	Rx X/o	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	.0.8	1.0
	1.0		+.0000	+.0000	+.0000	+.0000		0	0	0	0	0.0	0.0	0
_%	0.8	0001	+.0000	+.0000	+.0000	+.0000	+.0000	0	+.0000	 	 	+	 	+.0000
-`	0.6	0002	+.0001	+.0001	+.0000	+.0000	+.0000	0	+.0000	+.0000	+.0000	 -		+.0000
11	0.4	+.0040		+.0003	+.0002	+.0001	+.0001	0	+.0001	+.0001	+.0001	+.0001	+.0001	+.0001
%	0.2	+ .0482	+.0023	+.0013	+.0006	+.0002		 -	+.0005	+.0003	+.0001	0001	0002	0002
°	<u>Q.</u>	+.0073	0	+.0001	+.0003		+.0008	0	0	+.0006	+.0016	+.0028	+.0040	+.0041
ļ	1.0	0009	+.0073	+.0091	+.0514	+.0001	+.1410	+. 1925		T -	1	т		
1	0.8	0002	+ 0002		+.0002	+.0001	+.0000	0	0	0	0	0	0	0
74	0.6	+.0006	+.0006	+.0005	+.0004	+.0002	+.0001	0	+.0000	+.0000	+.0001	+. 0001	 .	+.0001
"	0.4	+.0129	+.0020	+.0013		+.0003	+.0000	0	+ .0004	+.0003	+.0002	+.0001	+.0002	+.0003
	0.2	+.0719	+.0047	+.0019	+.0004	0003	0004	0	+.0009	+ 0002	0005	0011		+.0000
% %	0	0212	0	+.0003	+.0007	+.0011	+.0015	0	0	+.0014	+.0037		1 .	4
		RxRy	0212	+.0365	+.0923	+.1295	+.1807	+.2166						1.0001
	1.0	0035	+.0002	+.0004	+.0004	+.0003	+.0002	0	0	0	0	0	0	0
3/8	0.8	+ .0010	+.0007	4 .0006	+.0005	+.0003	+.0001	0	+.0001	+.0001	+.0002	+.0002	+.0002	+ 0003
w,	0.6	+ . 00 39	+.0015	+.0011	+.0006	+.0003	+.0001	0	+.0003	+ . 0003	+.0003	+.0003	+.0002	+.0002
li li	0.4	+ .0207			+.0006	0000	0002	0	+.0006	+.0004	+.0000	0003	0005	0007
10	0.2	+ .0756	+.0051		0004	0008 +.0016	+.0020	0	+.0010	0002	0013	0021	0026	0030
%	0	+ .0221	0		+.0011	٥	0	+.0023	+.0055	+.0080	+.0102	8110.+		
ļ	 				+.1027	+.1249	+.1699							
	0.8			+.0008		0	+.0003	0	0	0	0	0		
2	0.6					+.0004	+.0001 0000	0	+.0005		+.0002	+.0001	+.0002 0000	
1	0.4				+ 0003	0003	0004	0	+.0008	+ .0003	0003	0008	+	0001 0014
"	0.2	+.0747			0008	0010	0007	0	+.0010	0007	0019	0026		0032
q/p	0	+.0201	0				+.0025	0	0					+.0140
		RX	+.0201	+.0795	+.1217	+.1402	+.1629	+.1705					_::_::.	
	1.0	0037	+.0017	+ .0014	8000.	+.0003	+ .0000	0	0	0	0	0	0	0
.4	0.8	+ .0068	+.0024	+ . 0014	+.0006	+.0001	~.0001	0	+.0005	+.0003	+.0002	+ 0000	0001	0002
3/4	0.6	+.0104	+.0031	+ .0014	0003	0002	0002	0	+.0006	+.0004	0000	0004	0007	0009
13	0.4					0006	0005	0	+.0008	0000	0009	~.0015 ·	0019	0021
9	0.2		+.0041			0008	0005	0	+.0008	0014	0024		+	- 0026
6	0	+ .0215		+.0012			+.0030	0	0	+.0061	+.0106	+.0133	+.0152	+.0167
-								+.1710			- 2 1			
	0.8				+.0005	0001 0002	0003 0003	0	+ 0006	+ .0003	+ 0001	- 0002	0003	00005
-	0.6					0004	0003	0		+ .0003		0007		0012
11	0.4	+.0246				0007	0004	0	+ 0008				- 0019	0021
q/o	0.2						0003	0	+.0007					0017
	٥.	+.0258		+.0016			+ .0034	0	0			+		+.0181
		Ry		+ .1252 +			1683	+.1704						$\neg \neg$
*	- 0	qq	1			oment = action =					**	R, R,		→ x
9	,,,,,,,,	Д ў .,	- l m ->-	- ρ						PO	W. SITIVE	SIGN C	ONVENT	ION

FIGURE 32.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load VI, 1/3 uniformly varying load.

					٨	Иx					N	l _y			
	y/b	Rx X/0	0	0.2	0.4	0.6	0.8	1.0	0	0.2	0.4	0.6	0.8	1.0	
	1.0	0000	+.0000	+.0000	+.0000	+.0000	+.0000	0	0	0	0	0	0	0	
<u>8</u> /	0.8	0000	+.0000	+.0000	+.0000	+.0000	+.0000	0	+.0000	+0000	+.0000	+.0000	+.0000	+.0000	
-	0.6	0001	+.0000	+ .0000	+.0000	+.0000	+.0000	0	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000	
- 0	0.4	0001	+.0001	+.0001	+.0000	+.0000	+.0000	0_	+.0000	+.0000	+.0000	+.0000	+.0000	+.0000	
q/p	0.2	+.0137	+.0006		+.0002	+.0001	+.0000	0	+.0001	+.0001	+ .0001	+.0000	_	+.0000	
0,	0	+.0050	0	+.0001	+.0002	+.0003	+.0004	0	0	+ .0004	+.0010	+.0016	+.0022	+.0027	
		R _X R _y	+.0050	+.0169	+.0463	+.0669	+.0961	+.1185	<u> </u>	1 .					
	1.0	0002	+.0000	+.0000	+.0000	+.0000	+.0000	0	0	0	0	+.0000	0	0	
1/4	0.8	0001	+.0000	+ .0000	+.0000	+.0000	+.0000	0	+.0000	+.0000	+.0000	+.0000	+.0000		
	0.6	0001	+ .0001	+.0001	+.0001	+.0001	+.0000	0	+.0000	+.0001	+.0001	+.0001	+.0001	+.0001	
"	0.4	+.0011	+ .0003	+.0005		0001	0001	0	+.0001	+.0001	0000	0001	0002	0003	
a/p	0.2	+.0202 0036	0	+.0003	+.0003		+.0006	0	0	+.0008	+.0017	+ 0025	+.0031	+.0036	
	L ů	R _X R _y	0036	+.0391	+ .0667	+.0815	+.0997	+.1086	l 	1.0000					
-	1.0	0007	+.0000	+.0001	+.0001	+.0001	+.0000	0	0	0	0	0	0	0	
_	0.8	+.0001	+.0001	+.0001	+.0001	+.0001	+.0000	0	+.0000	+.0000	+.0000	+.0000	+.0000	-	
3/8	0.6	+.0004	+ .0003	+.0002	+.0001	+.0001	+.0000	0	+.0001	+.0001	+.0001	+.0001	+.0001		
1	0.4	+.0026	+ .0006	+ .0003	+.0001	+ .0000	0000	0	+.0001	+.0001	+ .0001	+.0000	0000	0000	
1 11	0.2	+.0212	+.0013	+ .0003	0001	- 0002	0001	0	+.0003	+.0000	0002	0004	0005	0006	
%	0 +.0194 0 +.0002 +.0004 +.0006 +.0007 0 0 +.0011 +.0022 +.0030 +.0035 +. R _X R _Y +.0194 +.0511 +.0673 +.0740 +.0825 +.0846														
	R _X R _Y + . 0194 + . 0511 + . 0673 + . 0740 + . 0825 + . 0846														
-	R _X R _Y + .0194 + .0511 + .0673 + .0740 + .0825 + .0846														
	1. 00010 +.0001 +.0001 +.0001 +.0001 +.0000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0														
1/2	0.6	+.0010	+.0004	+.0003	+.0001	+.0000	+.0000	0	+.0001	+.0001	+.0001	+.0000	+.0000	+.0000	
	0.4	+.0033	+ .0007		+.0001	0000	0001	0	+.0001	+.0001	+.0000	0001	0901	0002	
"	0.2	+.0209	+.0012	+.0001	0002	0002	0001	0	+.0002	0001	0004	0005	0006	0007	
0/p	0	+.0202	0		+.0005		+.0008	0	0	+.0015	+.0027	+.0034	+.0039	+.0043	
		R _x R _y	+.0202	+.0591	+.0732	+.0780	+.0835	+.0843							
	1.0	0009	+.0003	+.0002	+.0001	+.0001	+.0000	0	0	0	0	0	. 0	0	
_	0.8	+.0010	+.0004	+.0002	+.0001	+.0000	0000	0	+.0001	+.0001	+.0000	+.0000	0000	0000	
3/4	0.6	+.0016	+.0005	+.0003	+.0001	0000	0000	0	+.0001	+.0001	+.0000	0000	000i	0001	
ii ii	0.4	+.0037	+.0008	+.0002	0001	0001	0001	0	+.0002	+.0000	0001	0002	- 0003	0003	
ا م	0.2	+.0204	+.0010	0001	0003	0002	0001	0	+.0002	0003	0005	0006	0006	0006	
0	0	+.0233	0	+.0005	+.0007	+.0008	+.0009	0	0	+.0023	+.0035	+.0041	+.0045	+.0048	
		Rx Ry	+.0233	+.0688	+.0787	+.0811	+.0838	+.0841							
	1.0	0003	+.0004	+.0003	+.0001	0000	0000	0	0	0	0	0	0	0	
	0.8	+.0014	+.0005	+.0002	+.0001	0000	0000	0	+.0001	+.0001	+.0000	0000	0000	0001	
-	0.6	+.0017	+.0006	+.0002	0000	0001	0001	0	+ .0001	+.0001	0000	0001	0001	0002	
"	0.4	+.0035	+.0007	+.0001	0001	0001	0001	0	+.0001		0002		0003	0003	
o/p	0.2	+.0206	+.0008	0002	0002	0002	0001	0	+.0002	0004	0006	0005		0005	
	0	+.0270	0	+.0006	+.0008	+.0009	+.0010	0	0	+.0028	+.0040	+.0045	+.0048	+.0050	
		Ry	+.0270	+.0739	+.0808	+.0822	+.0838	+.0839							
¥	0		ý. - x - ¥				(Coeffi (Coeffi				M× W	R _x R _y	M _y	- ×	
U			'.	.,						P	OSITIVE	SIGN C	ONVEN	TION	

Figure 33.—Plate fixed along two adjacent edges, moment and reaction coefficients, Load VII, 1/6 uniformly varying load.

	Г		Ī	Mx						1						
	У/ь	R. X/o	-	0.05	0.1		0.1	104	105	 _	0.05	101	My	T		T
\vdash	0.5	+.5055			 	+.0024	0.3 0226	0.4	0.5	0	0.05	0.1	0.2	0.3	0.4	0.5
	0.4	+.5068	+.0825	+		+.0022	0225	+		+		+	+.0002		0082	0093
1%	0.3	+.5060	+.0796		 		0219	+	+	+.0159	+	 	+	0055 0071	0088	
",	0.2	+.4778	+.0690	+.0470	+.0282	0004	0192	+	+		-	-	+	+	0132	0145
مّ ا	0.1	+.3316	+.0400	+.0254	+.0139	0017	0108	0155	0170	+.0080	+.0047	-	0033	+	0084	0090
9	0.05			+.0108	 		0026	+		+.0034	+.0026	+.0026	+.0044	+.0071	+.0094	+.0102
	<u> </u>	0513 R _x R _y	0	+.0005	_	+.0047	+.0076	+			+.0024	+.0078	+.0234	+.0381	+.0481	+.0516
-	0.5	+.5142	+.0815	0797 +.0573		+.2203	+.3559 0224	+.4352 0365	+	+		T	T		T	r
	0.4	+.5111	+.0797	+.0557		+.0011	0220		+	+.0159		+.0068	0012 0017	007I	0108	0121
%	0.3	+.4928	+.0728	+.0499		0000	0203	+		+.0146	 -	4	0031	0093	0132	0129 0145
1 11	0.2	+.4260	+.0568	+.0375	+.0217	0014	0159	0238		+.0114		+.0030		0096	0128	0139
مي ا	0.1	+.2350	+.0270			0011	0066	0092	0100	+.0054	+.0032		0006		0013	0012
0	0.05			+.0066			+.0003		+	+	+.0022	+.0034	+.0082	+.0135	+.0174	+.0188
	<u> </u>	0496 Rx Ry	0 0496	+.0005 0631		+.0049	+.0080				+.0025	+.0082	+.0247	+ 0399	+.0502	+.0538
\vdash	0.5	+.5143	+.0765	+.0526	+.0319	0001	0214	+.4382 0336	+.4638	 		1. 0054		1 2121	1	
	0.4	+.5045				0004	0207	0321	0358		+.0102	+.0054	0033 0037	0101 0104	0144	0159
%	0.3	+.4660		+.0429	+.0251	0012	0181	0274		+	+.0082		0045	0107	0147	0161 0159
H	0.2	+.3697	+.0462	+.0297	+.0166	0017	0127	0186	0204	+	+.0055) 	0039	0082	0106	0114
٥	0.1	+.1635	+.0191	+.0119	+.0065	0004	0037	0052	0056	+.0038	+.0025	+.0018	+.0022	+.0036	+.0050	+.0056
0	-	+.0150		+.0046	+.0030	+.0018	+.0020	+.0025	+.0028	+.0013	+.0021	+.0042	+.0110	+.0180	+.0231	+.0249
1 .	P	0454 R _x R _y	0	+.0005	+.0017	+.0050	+.0082	+.0102		0	+.0025	+.0083	+.0252	+.0408	+.0511	+.0547
\vdash	0.5	+.4999	0454 +.0686	0527 +.0457	+.0410	+.2277 0017	+.3616 0196	+.4394 0293	+.4648	1 0122						
	0.4	+.4845		+.0432	+.0248	0017	0196	0293	0324 0306	+.0137	+.0087		0055 0056	0128	0175	0191
3,4		+.4311		+.0357	+.0200	0022	0156	0227	- 0249				0054	0126 011 3	0171 0150	0186 0162
1 11	0.2	+.3179	+.0374	+.0235	+.0126	0019	0101	0142	0155		+.0043	+.0014	0033	0064	0081	0086
	0.1	+.1133	+.0140	+.0089	+.0049	+.0001	001B	0025	0026	+.0028	+.0021	+.0023	+.0045	+.0074		+.0107
q/و	0.05	0109					+.0031	+.0039	+.0043	+.0009	+.0021	+.0048	+.0129	+.0210	+.0268	+.0288
1	0	0412	0		+.0017		+.0082	+.0102	+.0109	0	+.0024	+ 0083	+.0254	+.0409	+.0511	+.0546
Н	$\overline{}$		0412 +.0592	\longrightarrow	+.0445	+.2305 0031	+.3626	+.4384	+.4629			1			T	
1 1					+.0193	0031	0172 0162	0245 0229	0267 0250	+.0118			0072	0146		0209
%					+.0153	0029	0131	0183	0198	+.0092			0070 0059	0139 0113	0183	0198 0157
	0.2	+.2736			+.0094	0020	0079	0107	0114	+.0060			0026	0047		0061
1 1	0.1	+.0798	+.0106	+.0068	+ .0037	+.0005	-,0005	0006	0006			+.0027				+.0139
1%			+.0031	+.0026	1200.+	+.0025		+.0047	+.0051	+.0006	+.0021	+.0051	+.0140	+.0226	+.0285	+.0306
	_0	0377				+.0050			+.0106	0	+.0024	+.0083	+.0251	+.0400	+.0497	+.0530
\vdash		R _x R _y	0377		+.0503				+.4546	1			r			
1 1				+	+.0156	0040 0039	0147 0137	0198 0184		+.0100		+.0007		0153		0213
-	\rightarrow		+		+.0112	0033	0109	0143	~.0157	-		+.0006	0078 0061	0143 0109	$\overline{}$	01 98 0147
"	0.2				+.0068	0020	0061	0078	0082			+.0007	0020	0033		0040
2			+.0082				+.0003	+.0006	+.0007	+.0016	+.0018	+.0028	+.0068	+.0112	+.0144	0156
	0.05	0316	+.0024	1200.+	+.0018	+.0026	+.0040	+.0050	+.0054	+.0005	+.0021	+.0052	+.0143	+.0229	+.0286 -	.0306
l	0	035 I				+.0049				0	+.0024	+.0082	+.0244	+.0382	+.0470 +	.0500
- 			0351	0316	+.0585	+.2373	+.3551	+.4189	+.4389							
į	-												Y			
						•			•	ent)(pa² ent)(pa	•	1	A. R.	R _y M,	/p	x
o#	m		- <i>/</i> ¥	– x	-D→	π.						W PO:	SITIVE :	SIGN CC	NVENTI	ON

FIGURE 34.—Plate fixed along four edges, moment and reaction coefficients, Load I, uniform load.

RESULTS 41

		Mx											My		.004200600066 .007601020111 .007200890095 .0024 +.0035 +.0039 .0269 +.0334 +.0356 .002900390043 .006000800087 .008301070114 .004500520054 .0053 +.0070 +.0076 .0247 +.0301 +.0319 .005500460050 .006700860092 .007800970103 .002500260026				
	У/ _b	R _X X/o	0	0.05	0.1	0.2	0.3	0.4	0.5	0	0.05	0.1	0.2	0.3	0.4	0.5			
	0.5	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0			
	0.4	+.1047	+.0162	+.0113		+.0002	0045	0072	0081	+.0032	+.0022	+.0013	0004	0017					
m	0.3	+.2066	+.0305	+.0209	+.0126	0001 0012	0086 0108	0134	0149 0176		+.0041		0014		_				
- 11	0.2	+.2574	+.0278	+.0167	+.0083	0012	0078	0160 0103	0111		+.0047	+.0018	0034			-			
q/p		+.1363	+.0135	+.0077	+.0036	0007	0024	0029	0030		+.0016			+.0024	+				
ا ا	0	0376	0	+.0004	+.0012	+.0034	+.0054	+.0067	+.0071	0	+.0019	+.0059		+.0269					
<u></u>		R _X R _Y	0376	0368	+.0540	+.1962	+.2885	+.3395	+.3557										
	0.5	0	0	0	0	0	0	0	0	0	0	0	0						
12/	0.4	+.1041	+.0142	+.0094	+.0054	0004	0041	0060	0067		+.0018		0013						
	0.3	+.2490	+.0282	+.0162	+.0090	0012 0022	~.007 3 ~.0082	0105 0111	0114		+.0030		0029 0045						
	0.1	+.1921	+.0176	+.0096	+.0041	0020	0046	0055	0058		+.0016	0002	0029	0045					
9/p	0.05		+.0077	+.0041	+.0017	0002	0005	0003	0002		+.0011								
	0	0355	0	+.0004						0	+.0019	+.0059	+.0161	+.0247	+.0301	+.0319			
—		R _X R _Y	0355	0159	+.0675		+.2707			<u> </u>				r _		-			
	0.5	0	0	0	0	0 - 0000	0 - 0033	00 4 5	0 0048	0	0	0	- 0010						
5/8	0.4	+.1763	+.0112			0009	0033 0057	±.0045 ±.0075			+.0012		0019 0037						
		+.2120			+.0049	0025	0060	0074	0077		+.0018	0005	0047	0078					
"	0.1	+.1495	+.0117			0016	0027	0029	0030	+.0023		0004	0019	0025					
9/p	0.05	+.0601	+.0049	+.0024	+.0009	+.0001	+.0004	+.0007	+.0009	+.0010	+.0008	+.0013	+.0037	+.0063	+.0080	+.0086			
	0	0324	0	+.0004		+.0029	-	+.0051	+.0054	0	+.0019	+.0056	+.0146	+.0215	+.0257	+.0271			
\vdash		R _x	0324	+.0006	+.0781	+ 1882	+.2497	+. 2798	+.2888				_	_					
	0.5	0 +.0860	+.0085	+.0048	+.0021	00II	0026	00 3 2	0 0033	+.0017	0 + 0008	0 0002	0021	0 0037	0 0046	0 00 49			
3/4	0.3	+. 1545	+.0144	+.0078	+.0031	0021	0043	0052	0053		+.0012	0005	0039	0065	0081	0086			
111	0.2	+.1807	+.0149	+.0075	+.0025	0025	0043	0048	0049	+.0030	+.0010	0010	0045	0069	0083	0087			
ام ا	0.1	+.1218	+.0082	+.0035	+.0008	0013	0016	0015	0015	+.0016	+.0004	0005	- .001 3	0013	0011	0010			
0	0.05	+.0459	+.0034	+.0014	+.0005		+.0007	+.0011	+.0012	+.0007		+.0014				+.0082			
	0	- 0294 Ry	0	+.0004	+.0011	+.0026		+.0043		0	+.0019	+.0053	+.0129	+.0183	+.0213	+.0223			
-	0.5	R _X O	0294 0	+.0 34 0	+.0861	+. 1894 0	+.2278 0	+.2491	+.2551 0	0	0	0	0	0	0	0			
	0.4	+.0758	+.0065	+.0033	+.0011	0011	0020	0022	0023	+.0013	+.0005	0004	0021	0034	~.0042	0045			
2/8	0.3		+.0108			0020	0033	0035		+.0022		0009	0038	0059	007ı	0075			
	0.2	+.1560	+.0111	+.0049	+.0011	0022	003 I	0032	0032	+.0022	+.0005	0012	0041	0059	0069	0072			
	0.1	+.1028	+.0060	+.0022	+.0002	0010	0010	0008	0007	+.0012	+.0002	0005	0008	0006	0003	0002			
0/p		+.0371	+.0024		+.0003		+.0008	+.0011	+.0012		+.0006	+.0013	+.0037	+.0057	+.0069	+.0073			
	0	0266 R _x R _y	0 0266	+.0004	+.0010		+.0031	+.0035	+.0036	0	+.001 9	+.0050	+.0112	+.0153	+.0175	+.0181			
	0.5	<u>*</u> 0	0	0	0	0	0	0	0	0	0	0	0	0	0	6			
	0.4	+.0670	+.0050			0011	0015	0016		+.0010		0005	0020	0031	0037	0039			
[-]	0.3	+.1190	+.0083	+.0036	+.0006	0019	0025	0025	0024	+.0017	+.0003	0011	0035	0052	0061	0064			
"	0.2	+. 1366	+.0084	+.0033	+.0002	0019	0023	0022	0021	_	+.0002	0013	0036	0050	0057	0059			
1%	0.1	+.0889		+.0014	0001	0007	0006	0004	0003		+.0001	0005	0006	0003		+.0001			
	0.05	0241	+.0018					+.0011	+.0011	+.0004	+.0005	+ 0046	+ 0098	+ 0128	+.0143	+.0148			
	Ť	Ry	0241	+.0311		+.1614	+.1877	+.1971	+. 1994										
-																			
	 -	- a	→										ļ						
	Moment = (Coefficient)(pa ²) Reaction = (Coefficient)(pa) R_x R_y R_y R_y										- x								
0	7777	1111111	1	. Х	p->-							PC	SITIVE	SIGN C	ONVENT	ION			

Figure 35.—Plate fixed along four edges, moment and reaction coefficients, Load-X, uniformly varying load, p=0 along y=b/2.

_	7		T -							T						
	V/	- x/	1	T	,	M _x			-	ļ			My			
<u> </u>	у/ _b		-	0.05	0.1	0.2	0.3	0.4	0.5	0	0.05	0.1	0.2	0.3	0.4	0.5
1	0.5	+.1988		+		+	+		-+	+.0032	+.0015	+.0002	0013	0015	0010	ō
\ w	0.4	+.1989	+	+.0073			+	†···	+		+ 0015	+	+	·		0
E/	0.2	+.1989				+ · ·	+		+	+	+.5014	+		0016	0010	0
"_	0 1	+.1858	+	+.0051				+	+		+.0013	+	+		+	0
1%	0.05	+.1327	+.0077	+	+	-	+	+	0		+ 0008	0004	+	-	+	0
	0	0137	0	+.0002	+.0006	+.0012	+	+.0008		1-0	+.0011	·		0007		0
	ļ	R _X R _y	0137	+.0307	+.0816	+.1278	+.1137	+.0649	0	1		1	11,5551	1.0000	1.0040	0
	0.5	+ 1991		+.0073		+	0076	004F	0	+.0032	+.0015	+.0002	0013	0016	0010	0
1/2	0.4	+.1992	+	+.0073			+		0	+ 0032	+.0014	+.0001	0013	0016		0
Į.	0.3	+.1994	+	+.0071		+		1				+.0000		0018	0011	0
 q	0.2		+.0147	+.0062				0040	+			0002		0021	0014	0
2	0.05			+	+	0016	0014		0			0007				0
-	0	0201	0			+.0014			0	+.0011			+.0002			0
L		R _X R _y	0201				+.1174		0	<u> </u>		11.0033	1.0072	+.0075	[+.0046]	
	0.5	+.1997	+.0160	+.0072	+.0008	0063	0075	0047	0	+.0032	+.0014	+.0001	- 0014	- 0017	- 0011	
	0.4	+.1997	+.0159	+.0071	+.0007	0063	0074	0046	0			+.0001	0015	0017	0011	0
5/8	0.3		+.0153			0061	0069	0043	0	+.0031		0001		0020		0
li i	0.2		+.0134			0053	0057	0034	0	+.0027	+.0009			0023	0014	0
م ا	0.1		+.0084			0030	0028	0016	0	+.0017			0015		0007	0
6			+.0040		0003	0009	0007	+	0			+.0004				0
	0	0232 R _x R _y	0 - 0272	+.0003		+.0016		+.0010	0	0	+.0015	+.0040	+.0078	+.0079	+.0048	0
-	_		+.0158			+.1345 0063	+.1188 0073		0							
			+.0155			0062	0071	0045	00			+.0000	+		0012	0
34			+.0146			0058	0064	0039	0	+.0029		0001 0003		0020	0013	0
i l			+.0122		0003	0047	0049	0029	0			0006		0022	0014 0014	0
	0. I	+. 1274	+.0068	+.0021	0006	+	0020		o	+.0014		0006	- +-		0003	-0
%	0.05	+.0581	+.0030	+.0008	0002	0004	0002	0000	0	-		+.0007				\dashv
Į.	0	0243	0	+.0003	+.0008	+.0016	+.0016	+.0010	0	1		+.0042			+.0049	-
	$\overline{}$	Rx Ry				+.1358	+.1193	+.0674	0							
ŀ			+.0154			0061	0069	0043	0	+.0031	+.0013	0001	0018 -	0022	0014	0
			+.0150	+		0060	0067	0041	0	+.0030			0019	0022	0014	0
%			+.0138			0054	0058	0035	. 0	+.0028					0015	0
11	$\overline{}$		+.0109		0005		0042	0024	0	+.0022					0012	
%			+.0023			0017	0015	0008	0				0003 +			_ 0
0)	\rightarrow	0245		+.0004			+.0001	+.0001	0		+.0005			.0031		
ı		Ry		+.0297				+.0673	0	0 1	+.0018	+.0044	+.0083 +	.0083	+.0050	
	$\overline{}$		+.0148				0064	0039	-	+.0030	F.0011	0003	0021	.0024	0016	0
[+.0143				0061	0037	0	+.0029					0016	
-			+.0128				0052	003ı	0	+.0026		0006 -			0015	-
			+.0098			0036		0020	0	+.0020 +			0019 -	.0019 -	0011	0
_ ` [_		+.0046					0005	0	+.0009 +		0001 4				0
۲			+.0018						<u> </u>			+.0014 +				0
⊢	• 	0241		+.0004 - +.0310 -		+.0017 - +.1366 -		+.0010	0	0]+	- 0018	+.0045	0084 +	.0083	1.0050	•
 ‡			.0241]	1.0310[0669	T. 1306]	r. 11901	+.0670	0							
Ĺ		n>	-4										Y			
		•	Ì										_ ∱ _		_	
1			2 1										И	٠.	<u>/</u> 6	
1												М	4	\		
3						- 1	Moment	= (Coeff	icient)(pa²)		-€	R.			
1			F			R	eaction	= (Coeff	icient)(pa)		•	1/1	R _v /	7	
													0	7/17	-	X
1												¥		√My		
1	m	m	,¥¥	-x								w	•	. 1		
<u> </u>	. , , , , , ,	///										POS	ITIVE S	IGN CO	NVENTIO	N
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Figure 36.—Plate fixed along four edges, moment and reaction coefficients, Load XI, uniformly varying load, p=0 along x=a/2.

Accuracy of Method of Analysis

The finite difference method is inherently approximate. A factor directly affecting its accuracy is the closeness of spacing, hence the number, of grid points. In obtaining the solutions presented in this monograph, a maximum number of points was used, consistent with the objectives of the study and the capacity of the available electronic calculator.

A few instances may be found where there appear to be irregularities in the orderly progression of the coefficients as the ratio a/b changes. Such instances are most likely to occur in the low values of the ratio where, to gain accuracy, the number of points used in the analysis was increased as a/b decreased. Although these inconsistencies are undesirable from an academic standpoint, they are not of sufficient magnitude to affect materially the usefulness of the results.

As a general check on the finite difference method, problems for which "exact" solutions are known have been computed. The results indicate that for spacings comparable to those used in this study, errors in the maximum moments may be of the order of five percent. Such accuracy is

considered to be satisfactory for design purposes. Percentage errors for small numerical values of the coefficients may, of course, be somewhat higher.

For Case 5 a comparison is given in Table 2

Table 2.—Comparison of Coefficients of Maximum Bending Moment at the Center of a Uniformly Loaded Rectangular Plate Fixed Along Four Edges

	Values of I	M _z /pa² from
b/a	Timoshenko ¹	Method of this Monograph ²
1. 1 1. 2 1. 3 1. 4 1. 6	$\begin{array}{c} -0.0264 \\ -0.0299 \\ -0.0327 \\ -0.0349 \\ -0.0381 \\ -0.0392 \end{array}$	$ \begin{array}{r} -0.0269 \\ -0.0301 \\ -0.0329 \\ -0.0352 \\ -0.0384 \\ -0.0395 \end{array} $
1.8	$ \begin{array}{c c} -0.0332 \\ -0.0401 \\ -0.0407 \end{array} $	-0.0333 -0.0404 -0.0410

 $^{^1}$ These values taken directly from page 228, Reference 1, with due regard for difference in sign conventions. 2 These values interpolated from the column for $\mu{=}0.3$ of the preceding table.

between values found on page 228 of Reference 1 and directly equivalent values obtained by the method of this monograph. In this particular case, the relative differences are, for the most part, less than one percent.

Comparisons have also been made with other existing results ² for full uniformly varying load and certain ratios of a/b. These indicated very

good agreement.

All coefficients have been computed to four decimal places for consistency and to indicate significant figures for many conditions which would have no significance to three decimal places. This should not be taken as an indication that the percentage accuracy is greater than noted above.

Appendix I

An Application to a Design Problem

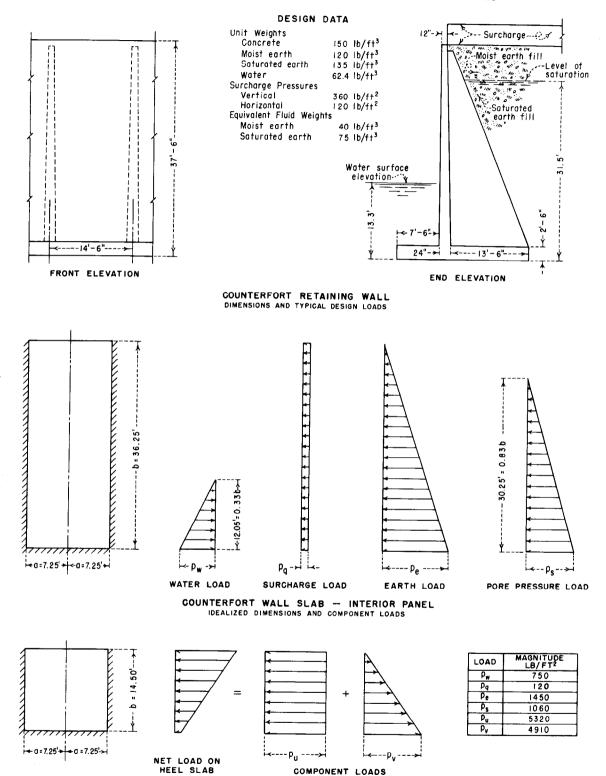
This appendix illustrates use of the tabulated coefficients by an application to a typical design problem. Figure 37 shows essential dimensions and typical loads acting on an interior panel of a counterfort retaining wall. Both wall and heel slabs approximate the condition of a plate fixed along three edges and free along the fourth. The variations in thickness of the wall slab and the relatively great thickness of the heel slab compared with its lesser lateral dimension are both, perhaps to some degree, in violation of basic assumptions. Ignoring these, however, is done with the conviction that results obtained in this manner are more nearly correct than what might be determined by other available methods.

Center line dimensions have been used for both slabs. The net loads, as determined from equi-

librium conditions, have been broken into components similar to certain of the typical Loads I through XI. These are illustrated together with a table of their numerical values in Figure 37.

It will be noted that for the wall slab, r=a/b=0.2. This requires interpolation on r for the various loads and in the case of p_s , interpolation both on r and the load. For the heel slab, r=a/b=1/2, and since both component loads act over the full area, no interpolation is required.

For illustrative purposes, moments have been computed along the assumed lines of support for both the wall and heel slabs. Where interpolation was required to obtain the moment coefficients, second degree interpolation was used. The moment coefficients and actual computed moments are given in Tables 3 through 6.



COUNTERFORT HEEL SLAB - INTERIOR PANEL IDEALIZED DIMENSIONS AND COMPONENT LOADS

COMPONENT LOADS

FIGURE 37.—Counterfort wall, design example.

Table 3.— M_{\star} for Heel Slab at Supports

Values	Values of pb²→		coefficients	Moments	(foot-kips)		
		1118.5	-1032.3			Total moment (foot-kips)	
<u>x</u> a	y b	p_{u}	₽v	$ m M_u$	M _v	,	
0	1.0	+0.0852	+0.0151	+95.30	-15.59	+79.7	
0	0. 6	+0.0807 $+0.0712$	+0.0216 $+0.0273$	+90.26 $+79.64$	$ \begin{array}{r} -22.30 \\ -28.18 \end{array} $	$+68.0 \\ +51.5$	
0	0. 4 0. 2	+0.0545 +0.0250	$+0.0277 \\ +0.0160$	+60.96 +27.96	-28.59 -16.52	$+32.4 \\ +11.4$	
0 0. 2	0	0 + 0.0019	$0 \\ +0.0014$	0 + 2.13	$0 \\ -1.45$	0 + 0.7	
0. 4 0. 6	0	$+0.0050 \\ +0.0080$	+0.0033 +0.0050	$+5.59 \\ +8.95$	-3.41 -5.16	+2.2 +3.8	
0.8 1.0	0	$+0.0100 \\ +0.0107$	+0.0061 +0.0065	+11.18 +11.97	$ \begin{array}{r} -6.30 \\ -6.71 \end{array} $	$+4.9 \\ +5.3$	

Table 4.—My for Heel Slab at Supports

Values	Values of pb²→		coefficients	Moments	(foot-kips)	
		1118.5	-1032.3			Total moment (foot-kips)
<u>x</u> a	$\frac{\mathbf{y}}{\mathbf{b}}$	Pu	pv	Mu	Μ _v	
0	1. 0	0	0	0	0	0
0	0.8	+0.0161	+0.0043	+18.01	-4.44	+13.6
0	0. 6	+0.0142	+0.0055	+15.88	-5. 68	+10.2
0	0. 4	+0.0109	+0.0055	+12.19	-5.68	+6.5
0	0. 2	+0.0050	+0.0032	+5.59	-3.30	+2.3
0	0	0	0	0	0	0
0. 2	0	+0.0094	+0.0068	+10.51	-7.02	+3.5
0.4	0	+0.0252	+0.0167	+28.19	17. 24	+11.0
0.6	0	+0.0399	+0.0252	+44.63	-26.01	+18.6
0.8	0	+0.0499	+0.0307	+55.81	-31.69	+24.1
1. 0	0	+0.0534	+0.0325	+59.73	-33.55	+26.2

MOMENTS AND REACTIONS FOR RECTANGULAR PLATES

TABLE 5.- Mx for Wall Slab at Supports

Valu	es of		Moment	coefficients			M	oments		
pb	2->	-985.5	157. 7	1905. 4	1392. 9			ot-kips)		Total moment (foot-kips)
X a	y b	p₩	Pq	p _e	p _e	M _w	Mq	M.	M,	(100t-kips)
0 0 0 0 0 0 0 2 0.2 0.4 0.6	1. 0 0. 8 0. 6 0. 4 0. 2 0 0 0	-0.0000 +0.0000 +0.0000 +0.0009 +0.0032 0 +0.0002 +0.0005 +0.0007 +0.0009	+0. 0133 +0. 0131 +0. 0134 +0. 0133 +0. 0103 0 +0. 0003 +0. 0009 +0. 0013 +0. 0016	+0.0012 +0.0028 +0.0054 +0.0079 +0.00079 0 +0.0003 +0.0007 +0.0011 +0.0014	+0.0004 +0.0012 +0.0034 +0.0068 +0.0075 0 +0.0003 +0.0006 +0.0011 +0.0014	+0.00 -0.00 -0.00 -0.89 -3.15 0 -0.20 -0.49 -0.69	+2. 10 +2. 07 +2. 11 +2. 10 +1. 62 0 +0. 05 +0. 14 +0. 21	+2. 29 +5. 34 +10. 29 +15. 05 +15. 05 0 +0. 57 +1. 33 +2. 10	+0. 56 +1. 67 +4. 74 +9. 47 +10. 45 0 +0. 42 +0. 84 +1. 53	+5. 0 +9. 1 +17. 1 +25. 7 +24. 0 0 +0. 8 +1. 8 +3. 2
1. 0	ő	+0.0010	+0 0018	+0.0015	+0.0014	-0. 99 -0. 99	+0. 25 +0. 28	+2.67 +2.86	+ 1. 95 + 2. 09	+4.0 +4.2

Table 6.—My for Wall Slab at Supports

Value	es of		Moment	coefficients			Me	oments		
pb²	→	985. 5	157. 7	1905. 4	1392. 9			ot-kips)		Total moment (foot-kips)
x a	у Б	p _w	Pq	p•	p ₄	M _w	Mq	M.	M.	(1000 21,00)
0	1. 0	0	0	0	0	0	0	0	0	0
0	0.8	-0.0000	+0.0026	+0.0005	+0.0002	+0.00	+0.41	+0.95	+0.28	+1.6
0	0. 6	+0.0000	+0.0027	+0.0011	+0.0007	-0.00	+0.43	+2.10	+0.98	+3.5
0	0. 4	+0.0002	+0.0026	+0.0016	+0.0014	-0.20	+0.41	+3.05	+1.95	+5.2
0	0. 2	+0.0006	+0.0020	+0.0016	+0.0015	-0.59	+0.32	+3.05	+2.09	+4.9
0	0	0	0	0	0	0	0	0	0	0
0. 2	0	+0.0011	+0.0015	+0.0014	+0.0014	-1.08	+0.24	+2.67	+1.95	+3.8
0. 4	0	+0.0025	+0.0041	+0.0036	+0.0036	-2.46	+0.65	+6.86	+5.01	+10.1
0. 6	0	+0.0036	+0.0066	+0.0056	+0.0055	-3.55	+1.04	+10.67	+7.66	+15.8
0.8	0	+0.0043	+0.0082	+0.0069	+0.0068	-4.24	+1.29	+13.15	+9.47	+19.7
1. 0	0	+0.0046	+0.0088	+0.0074	+0.0072	-4.53	+1.39	+14.10	+10.03	+21.0

Appendix II

The Finite Difference Method

Introduction

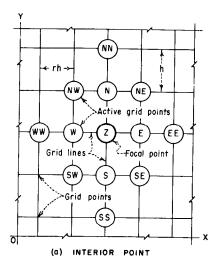
The bending of thin elastic plates or slabs subjected to loads normal to their surfaces has been studied by many investigators. have been solved by exact or approximate means, and these results are available. (See, for instance, have been solved by exact approximate methods are frequently difficult to apply except to structures where some symmetry exists and where a simple loading is used. The finite difference method, however, is readily adaptable to rectangular plates having any of the usual edge conditions and subjected to any loading.

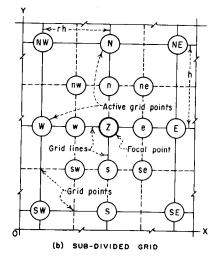
In Denmark, as early as 1918, N. J. Nielsen applied the finite difference method to the solution of plate problems. In his book 4 he has analyzed the problem in considerable detail and has given numerical solutions for a number of cases. H. Marcus published an excellent book 5 in Germany in 1924 on this subject in which he included numerous examples. In the United States, Wise, Holl, and Barton 6.7 8 have contributed to the literature of finite difference solutions for

rectangular plates, and Jensen ⁹ has extended the method to provide a useful tool in the analysis of skew slabs.

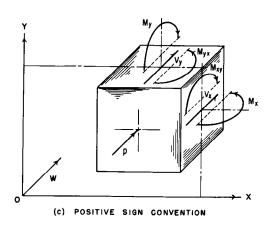
General Mathematical Relations

The partial differential equation, frequently called Lagrange's equation, which relates the rectangular coordinates, the load, the deflections, and the physical and elastic constants of a laterally loaded plate, is well known. Its application to the solution of problems of bending of plates or slabs is justified if the following conditions are met: (a) the plate or slab is composed of material which may be assumed to be homogeneous, isotropic, and elastic: (b) the plate is of /a uniform thickness which is small as compared with its lateral dimensions; (c) the deflections of the loaded plate are small as compared with its thickness. The additional differential expressions relating the deflections to the boundary conditions, moments, and shears are perhaps equally well known. (See, for instance,1.) They will therefore only be stated here, using the notation and sign convention shown in Figure 38.





GRID POINT DESIGNATION SYSTEM



```
Intensity of pressure, normal to the plane of the plate.
                    Lateral dimensions of the plate,
                   Lateral dimension in the y direction of the grid elements of the plate.
                    Ratio of lateral dimensions of the grid elements.
                    Deflection of the middle surface of the plate, normal to the XOY plane.
                    Rectangular coordinates in the plane of the plate.
Z, N, E, ... NE, NN
                    Designation of active grid points. Also used to represent the value
                      of the deflection of the plate at the point so lettered.
                    Designation of additional points on sub-divided grid.
                    Subscripts used to indicate directions normal and tangential to an edge.
         Mx, My
                    Bending moment per unit length acting on planes perpendicular to the x and y axes respectively.
                    Twisting moment per unit length in planes perpendicular to the x and y axes respectively.
                    Shearing force per unit length acting normal to the plane of the plate, in planes normal to
                     the x and y axes respectively.
                   Shearing reactions per unit length acting normal to the plane of the plate, in planes normal to
        Rx, Ry
                     the x and y axes respectively.
                   Concentrated load acting at a grid point; positive in the same direction as p.
                   Concentrated reaction acting at a supported grid point; positive direction opposite to that of \mathfrak{p}.
                   Young's modulus for the material of the plate
                   Moment of inertia per unit length of a section of the plate.
                   Poisson's ratio for the material of the plate.
                   Flexural rigidity per unit length of the plate; D = EI/(1-\mu^2).
                   Difference quotient operator: \nabla^4 w = \frac{\Delta^4 w}{\Delta x^4} + 2 \frac{\Delta^4 w}{\Delta x^2 \Delta y^2} + \frac{\Delta^4 w}{\Delta y^4}
```

NOTATION

FIGURE 38.—Grid point designation system and notation.

APPENDIX II 51

Partial differential equation:

$$\frac{\partial^4 \mathbf{w}}{\partial \mathbf{x}^4} + 2 \frac{\partial^4 \mathbf{w}}{\partial \mathbf{x}^2 \partial \mathbf{y}^2} + \frac{\partial^4 \mathbf{w}}{\partial \mathbf{y}^4} = \frac{\mathbf{p}(\mathbf{x}, \mathbf{y})}{\mathbf{D}}.$$
 (1)

Fixed edge conditions:

$$w=0,$$
 (2.01)

$$\frac{\partial \mathbf{w}}{\partial \mathbf{n}} = 0$$
 (2.02)

Hinged edge conditions:

$$w=0,$$
 (3.01)

$$\frac{\partial^2 \mathbf{w}}{\partial \mathbf{n}^2} + \mu \, \frac{\partial^2 \mathbf{w}}{\partial \mathbf{t}^2} = 0 \cdot \tag{3.02}$$

Free edge conditions:

$$\frac{\partial^2 \mathbf{w}}{\partial \mathbf{n}^2} + \mu \frac{\partial^2 \mathbf{w}}{\partial \mathbf{t}^2} = 0, \tag{4.01}$$

$$\frac{\partial^3 \mathbf{w}}{\partial \mathbf{n}^3} + (2 - \mu) \frac{\partial^3 \mathbf{w}}{\partial \mathbf{n} \partial \mathbf{t}^2} = 0$$
 (4.02)

Free corner conditions:

$$\frac{\partial^2 \mathbf{w}}{\partial \mathbf{n}^2} = 0$$
 (both directions), (5.01)

$$\frac{\partial^3 \mathbf{w}}{\partial \mathbf{n}^3} + (2 - \mu) \frac{\partial^3 \mathbf{w}}{\partial \mathbf{n} \partial \mathbf{t}^2} = 0$$
 (both directions), (5.02)

$$\frac{\partial^2 \mathbf{w}}{\partial \mathbf{n} \partial \mathbf{t}} = 0. \tag{5.03}$$

Bending moments:

$$M_x = D \left[\frac{\partial^2 w}{\partial x^2} + \mu \frac{\partial^2 w}{\partial v^2} \right], \quad (6.01)$$

$$M_{y} = D \left[\frac{\partial^{2} w}{\partial v^{2}} + \mu \frac{\partial^{2} w}{\partial x^{2}} \right]. \tag{6.02}$$

Twisting moments:

$$M_{xy} = M_{yx} = D(1 - \mu) \frac{\partial^2 w}{\partial x \partial y}.$$
 (7)

Shears:

$$V_x = -D \left[\frac{\partial^3 w}{\partial x^3} + \frac{\partial^3 w}{\partial x \partial y^2} \right],$$
 (8.01)

$$V_{y} = -D \left[\frac{\partial^{3} w}{\partial y^{3}} + \frac{\partial^{3} w}{\partial x^{2} \partial y} \right]$$
 (8.02)

In the above expressions the partial derivatives

with respect to n indicate rates of change in a direction normal to the edge, and those with respect to t indicate rates of change tangential to the edge.

A solution to any specific problem consists of determining a deflection surface which satisfies the basic equation (1), and the appropriate sets of boundary conditions (2.01) through (5.03). The moments and shears required for design purposes may then be computed from (6.01) through (8.02).

In general, it is difficult to obtain an analytical expression for a deflection surface which satisfies all of these conditions. If, however, an approximate solution is acceptable, it is always possible in analyzing a rectangular plate to determine a set of deflections for a finite number of discrete points such that approximate relations corresponding to (1) through (5.03) are satisfied. From these deflections it is possible to compute moments, reactions, and shears at the selected points, using relations similar to (6.01) through (8.02).

The approximate relations referred to above are obtained by replacing the partial derivatives by corresponding finite difference quotients. Such relations are simplest if the discrete points determined by values of the independent variables are equally spaced with respect to both variables. However, in this application it will be advantageous for the relations to be developed on the more general basis of having the equal spacing in one coordinate direction bear a given ratio to the spacing in the perpendicular direction.

Figure 38(a) represents a portion of the interior of a plate subdivided by grid lines into rectangular grid elements. The grid lines are spaced h units apart in the y direction and rh units apart in the x direction. The intersections of the grid lines will be referred to as grid points. Certain of these, lettered for identification, will be spoken of as active points, and the central point of the active group will be called the focal point. For simplicity in writing the equations, the identifying letters for each active point will also be used to represent the value of the deflection, w, of the middle surface of the plate at that point. The double letters refer in every case to the deflection at the individual point so lettered; they do not indicate products of deflections at points designated by only one letter.

Based on the usual methods of finite differences, 10 the difference quotient relations required in this development can be written directly and are given below. All of the difference quotients are given with reference to the focal point, lettered Z.

$$\frac{\Delta w}{\Delta x} = \frac{1}{2rh} (E - W), \qquad (9.01)$$

$$\frac{\Delta^2 w}{\Delta x^2} = \frac{1}{r^2 h^2} (E - 2Z + W), \qquad (9.02)$$

$$\frac{\Delta^3 w}{\Delta x^3} = \frac{1}{2r^3h^3} (EE - 2E + 2W - WW), \quad (9.03)$$

$$\frac{\Delta^4 w}{\Delta x^4} = \frac{1}{r^4 h^4} (EE - 4E + 6Z - 4W + WW), (9.04)$$

$$\frac{\Delta w}{\Delta y} = \frac{1}{2h} (N - S), \qquad (9.05)$$

$$\frac{\Delta^2 w}{\Delta v^2} = \frac{1}{h^2} (N - 2Z + S), \qquad (9.06)$$

$$\frac{\Delta^3 w}{\Delta y^3} = \frac{1}{2h^3} (NN - 2N + 2S - SS), \qquad (9.07)$$

$$\frac{\Delta^4 w}{\Delta y^4} = \frac{1}{h^4} (NN - 4N + 6Z - 4S + SS), \quad (9.08)$$

$$\frac{\Delta^2 w}{\Delta x \Delta y} = \frac{1}{4 \text{rh}^2} \text{ (NE-NW+SW-SE), (9.09)}$$

$$\frac{\Delta^{3}w}{\Delta x^{2}\Delta y} = \frac{1}{2r^{2}h^{3}} (NE - 2N + NW - SE + 2S - SW),$$
(9.10)

$$\frac{\Delta^3 \mathbf{w}}{\Delta \mathbf{x} \Delta \mathbf{y}^2} = \frac{1}{2 \mathrm{rh}^3} \text{ (NE} - 2 \mathrm{E} + \mathrm{SE} - \mathrm{NW} + 2 \mathrm{W} - \mathrm{SW}),$$
(9.11)

$$\frac{\Delta^{4}w}{\Delta x^{2}\Delta y^{2}} = \frac{1}{r^{2}h^{4}} (NE - 2E + SE - 2N + 4Z - 2S + NW - 2W + SW). (9.12)$$

The approximate counterparts of the basic relations (1) through (8.02) may now be written. For instance if ∇^4 w is used to represent the difference quotient equivalent to the left-hand member of equation (1), and the partial derivatives are replaced by their corresponding difference quotients, (9.04), (9.08), and (9.12), there results:

$$\nabla^{4}w = \frac{1}{r^{4}h^{4}} [EE + WW + r^{4}(NN + SS) + 2r^{2}(NE + SE + SW + NW)]$$

$$-4(1+r^2)(E+W)-4r^2(1+r^2)(N+S)$$

 $+2(3+4r^2+3r^4)Z$]. (10)

This may be considered as an operator, and the portion within the brackets can be conveniently portrayed as an array of coefficients. This expression, multiplied by h⁴, is shown in array form at (a) of Figure 39. Each element of the array represents the coefficient of the deflection of one of the active grid points in a group similar to that shown at (a) of Figure 38. The location of the coefficients in the array is congruent to the physical locations of the points and the heavily outlined coefficient applies at the focal point—the point for which the relation is to be determined.

Since the solution deals with discrete points, the distributed load intensity p in the right-hand member of (1) is replaced by an average intensity P/rh^2 at each of the interior grid points. Here P represents a concentrated load whose magnitude at any grid point is a function of the distribution of p on the four adjoining grid elements. If each of these elements is considered as an infinitely rigid plate supported at its four corners, then the force P_z , at the focal point, is equal in magnitude and opposite in direction to the sum of the reactions at all corners common to Z. This can be expressed mathematically as:

$$P_z = P_{zNE} + P_{zSE} + P_{zSW} + P_{zNW}$$
 (11)

in which $P_{z_{NE}}$ represents the contribution from the grid element Z-N-NE-E and similarly for the other right-hand members. Thus it is seen that the concentrated loads P_z are the static equivalent of p.

It can be shown, if p varies linearly—a usual condition for structures—and if this variation is constant over the four grid elements adjoining any focal point Z, that the magnitude of the statically equivalent average load is:

$$P_z/rh^2 = (1/6)(p_N + p_E + p_S + p_W + 2p_z),$$
 (12)

where p_N represents the intensity of p at point N, etc.

The approximate counterpart of (1) may now be written:

$$\nabla^4 \mathbf{w} = \frac{\mathbf{P_z}}{\mathbf{Drh^2}}.$$
 (13)

APPENDIX II

Multiplying both sides of (13) by h⁴ and replacing ∇⁴w by the deflections as given by (10) leads to:

$$\frac{1}{r^{4}}[EE+WW+r^{4}(NN+SS) + 2r^{2}(NE+SE+SW+NW) - 4(1+r^{2})(E+W)-4r^{2}(1+r^{2})(N+S) + 2(3+4r^{2}+3r^{4})Z] = \frac{P_{z}}{rh^{2}} \frac{h^{4}}{D}. \quad (14)$$

This is the general load-deflection relation for an interior point. It is written at (a) of Figure 39 in the convenient array form previously described. This general form of the equations has been used for the special cases which include the boundary conditions and, in fact, for all of the relations connecting the deflections with load, moments, reactions, and shears. These load-deflection equations establish a linear relation between the load at the focal point and the unknown deflections of the plate at that and the other active grid points. It is these linear equations which are to be solved simultaneously to determine the approximate deflections of the plate at the grid points.

Equation (14) may be derived directly by a second method which considers equilibrium of certain elements of the plate. Referring to the subdivided grid of Figure 38(b), consider the rectangular element ne-se-sw-nw with center at Z. Equilibrium of forces normal to the plate requires that

$$(V_{x_a}-V_{x_w})h+(V_{v_a}-V_{v_a})rh+P_z=0.$$
 (15)

For the similar element with center at e, equilibrium of moments about the center line ne-se requires that

$$\begin{split} (M_{x_{\rm E}} - M_{z_{\rm Z}})h + (M_{y_{x_{\rm ne}}} - M_{y_{z_{\rm se}}})rh \\ + (V_{x_{\rm E}} + V_{x_{\rm Z}}) \; \frac{rh^2}{2} = 0 \cdot \end{split}$$

However, if the elements are sufficiently small,

$$\frac{1}{2}\left(\mathbf{V_{x_E}} + \mathbf{V_{x_Z}}\right)$$

may be replaced with V_{x_a} so that

$$(M_{x_E}-M_{x_Z})h+(M_{y_{x_{n_e}}}-M_{y_{x_{g_e}}})rh+V_{x_e}rh^2=0.$$
(16.01)

In like manner for elements with centers at w, n, and s:

53

$$\begin{split} (M_{x_{Z}}-M_{x_{W}})h+(M_{yx_{n_{W}}}-M_{yx_{n_{W}}})rh+V_{x_{W}}rh^{2}=0,\\ (16.02)\\ (M_{y_{N}}-M_{y_{Z}})rh+(M_{xy_{n_{e}}}-M_{xy_{n_{W}}})h+V_{y_{n}}rh^{2}=0,\\ (16.03)\\ (M_{y_{Z}}-M_{y_{N}})rh+(M_{xy_{n_{e}}}-M_{xy_{n_{W}}})h+V_{y_{e}}rh^{2}=0. \end{split}$$

If equations (15) and (16.01) through (16.04) are combined to eliminate the shears, noting at the same time that $M_{xy}=M_{yx}$, there results

$$\begin{split} \frac{1}{r} \left(M_{x_{E}} - 2M_{x_{Z}} + M_{x_{W}} \right) + 2(M_{xy_{n_{e}}} - M_{xy_{n_{w}}} + M_{xy_{s_{w}}} \\ - M_{xy_{s_{e}}} \right) + r(M_{y_{N}} - 2M_{y_{Z}} + M_{y_{S}}) = P_{z}. \end{split} \tag{17}$$

An approximation to each moment in terms of deflections is obtained if the partial differentials of the definitions (6.01); (6.02), and (7) are replaced by their proper difference quotients corresponding to (9.02), (9.06), and (9.09). For instance,

$$M_{x_{Z}} = \frac{D}{r^{2}h^{2}} [E - 2Z + W + \mu r^{2}(N - 2Z + S)]$$
 (18)

and

$$M_{xy_{n_e}} = \frac{D(1-\mu)}{rh^2} [NE-N+Z-E]$$
 (19)

Substituting these and corresponding relations for the other moments into (17), and multiplying both sides by h^2/rD gives

$$\begin{aligned} &\frac{1}{r^4} \left(WW - 4W + 6Z - 4E + EE \right) + \frac{2}{r^2} \left(NW - 2N \right. \\ &\quad + NE - 2W + 4Z - 2E + SW - 2S + SE \right) \\ &\quad + (NN - 4N + 6Z - 4S + SS) = \frac{P_z h^2}{rD} \end{aligned}$$

which, with some rearrangement, is the same as (14).

This second method is easily adapted to deriving expressions involving nonuniform spacings, moment-free boundaries, etc. It was applied to obtain all of the load-deflection arrays shown in Figures 39 through 59, which were required in the solution of the problems covered by this monograph.

Where boundary conditions involve a reaction, the load P may be replaced by the net load, (P-R), which is the difference between load and reaction. Note that R represents a concentrated force whose positive direction is opposite to that of p. R_x and R_y , on the other hand, represent intensities of shearing reactions whose positive directions conform to V_x and V_y .

Relations connecting the deflections with moments and with shears are given in Figures 60 through 64. It should be noted that shears computed by finite difference methods are inherently less accurate than moments. This is because the shears are functions of odd numbered difference quotients which are determined by a grid spacing double the value found in the even numbered quotients which define the moments.

Application to Plate Fixed Along Three Edges and Free Along the Fourth

As an example of the use of this general method, its application to the problem of a plate fixed along three edges and free along the fourth is given below. The a/b ratio of 1/4 has been used to illustrate use of the 20 supplementary equations. Loads I, II, and IV only are included.

The plate is divided into grid elements and the grid points numbered systematically for identification. Layout of Plate, Figure 66, shows the method used in this case. Because of symmetry of the plate and loading about the line x=a, points which are symmetrical about this line will have equal deflections and are, therefore, numbered alike. This reduces considerably the number of unknown deflections to be determined.

With r=1/4 and $\mu=0.2$, the left-hand side of each of the load-deflection relations yields an array of numerical coefficients corresponding to the type of point it represents. These values have been computed for typical points and they are shown in Figure 65. They are used in writing the left-hand members of the simultaneous equations. Solution of these equations determines the deflections.

One equation must be written for each grid point having an unknown deflection. The equation corresponding to any point is formed as follows:

a. Select the array of load-deflection coefficients having edge conditions and

- spacings which correspond to those of the given point.
- b. Orient the focal point of this array at the given point.
- c. Multiply the unknown which represents the deflection of each active grid point by the corresponding coefficient.
- d. Equate the sum of these products to the load term for the given point.

For example, for Point 45 the array at (b) of Figure 65 must be used in order that the free edges correspond properly. Then, following the procedure outlined above, the left-hand member of the equation for Point 45 is

$$+256w_{25}+32w_{34}-1088w_{35}+28.8w_{36}+w_{43}$$
 $-68w_{44}+(1669+256)w_{45}-59.6w_{46}$
 $+32w_{54}-1088w_{55}+28.8w_{56}$

Noting that $R_z=0$ along the free edge it is seen that in this case the general expression for the right-hand terms is always $(P_z/rh^2)(h^4/D)$. Since these load terms are to be expressed as coefficients of ph^4/D , it remains to evaluate the P_z/rh^2 in terms of p for each point and each loading. At Point 45 the right-hand members for Loads I and IV may be obtained by direct application of (12). However, a discontinuity occurs in the magnitude of Load II within the grid elements adjoining Point 45. For this reason, the more general method expressed by (11) must be employed.

In particular for Load II, the elements 45–35–36–46 and 45–46–56–55 carry no load, and accordingly they make no contribution to P_{45} . The elements 45–44–34–35 and 45–55–54–44 each carry an equal portion of the uniform load. Under the assumptions leading to (11) it is found, by statics, that the contribution of each of these elements to P_{45} is $ph^2/144$. Hence, $P_{45}=ph^2/72$ and $P_{45}/rh^2=p/18$.

The complete set of 30 equations and the right-hand (load) terms are shown as two matrices in Figure 66. Simultaneous solution of the equations establishes a set of deflections for each of the 30 grid points, corresponding to each load. These results are tabulated in the upper portion of Figure 67.

The 20 supplementary equations used to determine the deflections of the row of points at $y=\frac{1}{4}h$ are set up in a similar manner. Equations are

written for each point of the 3-, 2-, 1-, and 7-rows (see Figure 68). However, in writing equations for the 3- and 2-rows use is made of the previously computed deflections for the 4- and 5-rows. In addition, the solution of the 20 equations gives new and improved values of deflections for the 3-, 2-, and 1-rows. For Point 42, for example, the array (f) of Figure 65 is used to conform with the spacing of the grid points involved. The equation for Load I is

$$\begin{aligned} -28w_{21} + 210w_{22} + 10w_{23} + 176w_{31} - 936w_{32} \\ -8w_{33} + \frac{64}{3}w_{47} - 364w_{41} + \frac{5057}{3}w_{42} \\ +176w_{51} - 936w_{52} - 8w_{53} = \frac{3}{4}\frac{ph^4}{D} - w_{44}. \end{aligned}$$

Substituting for Point 44, its deflection as determined from the 30 equations gives, for the right-hand member

$$(0.75 - 0.100572) \frac{ph^4}{D} = 0.649428 \frac{ph^4}{D}$$

The complete set of 20 equations for Loads I, II, and IV is given in Figure 68. Solution of these gives the deflections shown on the lower portion of Figure 67. Where improved values of the deflection were obtained, the former ones have been discarded as indicated in the figure. Comparison of old and new values shows that they approach closely for the points where y/b=0.4.

Having determined the deflections, reactions and moments may be computed by operating upon the deflections with the appropriate relations, typical samples of which are given in Figure 69. These numerical arrays were obtained similarly to those for the load-deflection relations, by inserting numerical values for r and μ in the proper general expressions of the referenced figures.

To illustrate the method of computation of reactions and moments, an example of each (Load I, a/b=1/4) is given below. At Point 30, for instance, using array (f) of Figure 69, the reaction is:

$$R_{30} = P_{30} + \frac{D}{h^2} (-32w_{27} - 16w_{31} + 128w_{37} - 32w_{47}).$$

Substituting numerical values for P₃₀ and the various deflections, this becomes

$$\begin{split} R_{30} &= 0.03125 ph^2 + \left(\frac{D}{h^2}\right) \left(\frac{ph^4}{D}\right) \\ & [-(32)(0.004944) - (16)(0.021325) \\ & + (128)(0.007860) - (32)(0.009833)] \\ & = (0.03125 + 0.192016) ph^2 = 0.223266 ph^2. \end{split}$$

This represents a concentrated force acting at Point 30. Assuming that it is uniformly distributed over a distance rh, it can be expressed as an average shearing reaction per unit length

$$R_{y_{30}} = R_{30}/rh = 0.893064ph$$

or in terms of b

$$R_{y_{30}} = 0.178613 pb$$

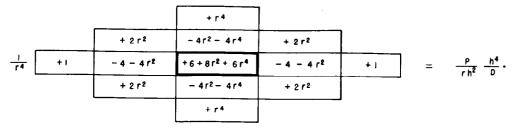
which is in the units used in Figures 1 through 33. Similarly, for example, the bending moment M_x at Point 23 is computed using array (g) of Figure 69. Thus

$$M_{x_{23}} = \frac{D}{h^2} (16w_{13} + 0.2w_{22} - 32.4w_{23} + 0.2w_{24} + 16w_{33}).$$

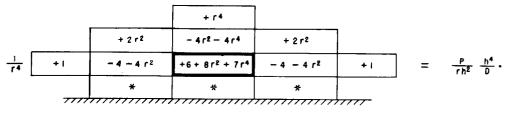
Again inserting numerical values

$$\begin{split} \mathbf{M_{x_{23}}} = & \left(\frac{\mathbf{D}}{\mathbf{h}^2}\right) \left(\frac{\mathbf{p}\mathbf{h}^4}{\mathbf{D}}\right) [(16)(0.015283) \\ & + (0.2)(0.029914) - (32.4)(0.043935) \\ & + (0.2)(0.046526) + (16)(0.073156)] \\ & = 0.006818\mathbf{p}\mathbf{h}^2 = 0.000273\mathbf{p}\mathbf{b}^2. \end{split}$$

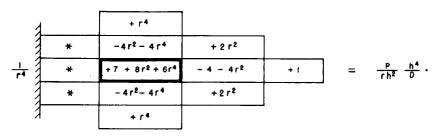
Upon completion of computation of the reactions, a partial check of the solution may be obtained from equilibrium considerations. For Load I, a/b=1/4, the total load on one-half of the plate is p(5h)(5h/4)=6.25 ph². The summation of the R/ph² column of Figure 70 should agree with this, and it is seen to be in error by something less than 0.015 percent.



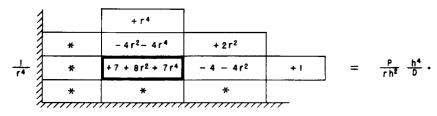
(a) INTERIOR POINT



(b) POINT ADJACENT TO A FIXED X-EDGE



(c) POINT ADJACENT TO A FIXED Y-EDGE



(d) POINT ADJACENT TO A FIXED CORNER

NOTES

FIGURE 39.—Load-deflection relations, Sheet I.

	+(2-µ)r2	$-2(2-\mu)r^2-2r^4$	+2(1-µ)r2		
+1	-4 - 4r2	+5 + 812 + 514		=	$\frac{P}{rh^2} \frac{h^4}{D}$.
<u> </u>	+ 2 r ²	-4r2 - 4r4	+(2-µ)r2		7 11- 0
		+ r4			

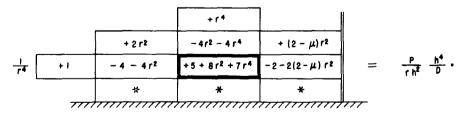
(0) POINT ADJACENT TO A MOMENT-FREE CORNER

-		+(2-µ) r2	-2(2-H)r2-2r4	+ (2 - μ) r²			
1 r4	+1	-4 - 4r2	+6 + 8r2 + 5r4	-4-452	+1	=	P h4
		+2 12	-4r2 - 4r4	+ 2 r ²			,
			+ r4		•		

(b) POINT ADJACENT TO A MOMENT-FREE X-EDGE

(c) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

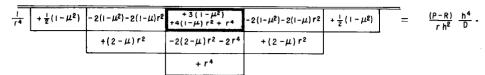
(d) POINT ADJACENT TO A MOMENT-FREE X-EDGE AND A FIXED Y-EDGE



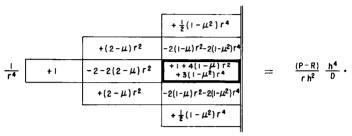
(e) POINT ADJACENT TO A MOMENT-FREE Y-EDGE AND A FIXED X-EDGE

Note.—For general notes see Figure 39.

FIGURE 40.—Load-deflection relations, Sheet II.



(4) POINT ON A MOMENT-FREE X-EDGE



(b) POINT ON A MOMENT-FREE Y-EDGE

$$\frac{1}{r^4} + \frac{1}{2}(1-\mu^2) - 2(1-\mu^2) - 2(1-\mu)r^2 + \frac{1}{2}\frac{(1-\mu^2)}{+4(1-\mu)r^2 + r^4} - (1-\mu^2) - 2(1-\mu)r^2 \\
+ (2-\mu)r^2 - 2(2-\mu)r^2 - 2r^4 + (2-\mu)r^2 \\
+ r^4$$

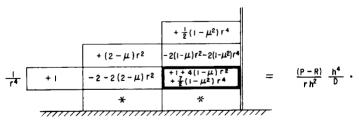
(c) POINT ON A MOMENT-FREE X-EDGE ADJACENT TO A MOMENT-FREE Y-EDGE

	+ (2 - \mu) r2	$-2(1-\mu)r^2-(1-\mu^2)r^4$		
+1	-2-2(2-µ)r²	$+1 + 4(1 - \mu) r^2 + \frac{5}{2}(1 - \mu^2) r^4$	=	$\frac{(P-R)}{rh^2} \frac{h^4}{D} \cdot$
	+(2 - \mu) r2	-2(1-µ)r2-2(1-µ2)r4		
		$+\frac{1}{2}(1-\mu^2)r^4$		

(d) POINT ON A MOMENT-FREE Y-EDGE ADJACENT TO A MOMENT-FREE X-EDGE

+ 3	*	$+\frac{7}{2}(1-\mu^2)$ + 4(1-\mu)r^2+r^4	-2(1-\mu^2)-2(1-\mu) r2	$+\frac{1}{2}(1-\mu^2)$		$\frac{(P-R)}{rh^2}\frac{h^4}{D}.$
		-2(2-µ)r2-2r4			l	
		+ r4				

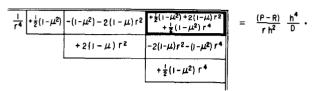
(e) POINT ON A MOMENT-FREE X-EDGE ADJACENT TO A FIXED Y-EDGE



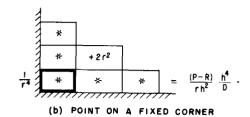
(f) POINT ON A MOMENT-FREE Y-EDGE ADJACENT TO A FIXED X-EDGE

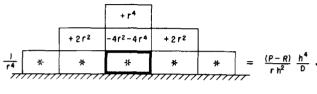
Note.—For general notes see Figure 39.

FIGURE 41.—Load-deflection relations, Sheet III.

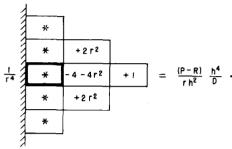


(0) POINT ON A MOMENT-FREE CORNER

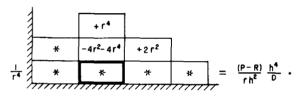




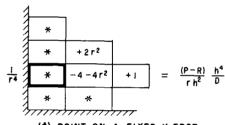
(c) POINT ON A FIXED X-EDGE



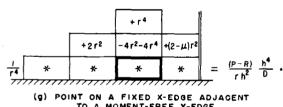
(d) POINT ON A FIXED Y-EDGE



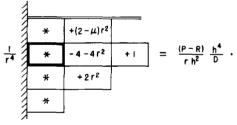
(e) POINT ON A FIXED X-EDGE ADJACENT TO A FIXED CORNER



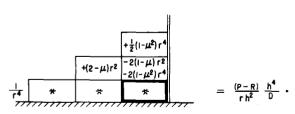




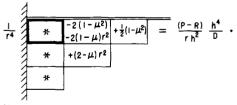
(g) POINT ON A FIXED X-EDGE ADJACENT TO A MOMENT-FREE Y-EDGE



(h) POINT ON A FIXED Y-EDGE ADJACENT TO A MOMENT-FREE X-EDGE



(i) POINT ON A FIXED X- MOMENT-FREE Y-CORNER

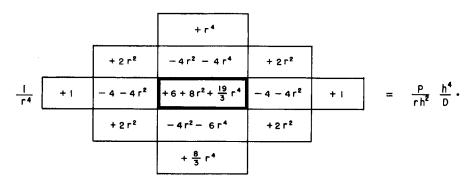


()) POINT ON A FIXED Y- MOMENT-FREE X-CORNER

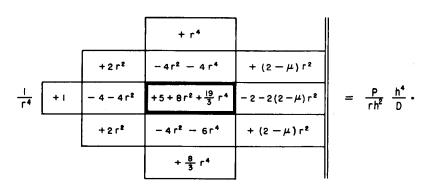
Note.—For general notes see Figure 39.

FIGURE 42.—Load-deflection relations, Sheet IV.

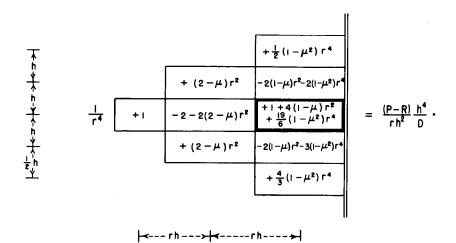




(a) INTERIOR POINT



(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE



(c) POINT ON A MOMENT-FREE Y-EDGE

Note.—For general notes see Figure 39.

Figure 43.—Load-deflection relations, vertical spacing: 3 at h; 1 at h/2, Sheet V.

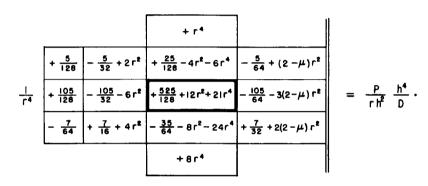
+ n + + n + n + n + n +

			+ r4				
	+ 5 128	$-\frac{5}{32} + 2r^2$	$+\frac{15}{64}-4r^2-6r^4$	$-\frac{5}{32} + 2r^2$	+ 5 128		
<u> </u>	+ 105	$-\frac{105}{32}-6r^2$	+ \frac{515}{64} + 12\Gamma^2 + 2 \Gamma^4	$-\frac{105}{32}-6r^2$	+ 105	=	$\frac{P}{rh^2} \frac{h^4}{D}$.
	- 7 64	+ 7/16 + 4 r2	$-\frac{21}{32}-8r^2-24r^4$	$+\frac{7}{16}+4r^2$	- 7 64		
			+814				

├<--rh-->├<---rh--->├<---rh--->├

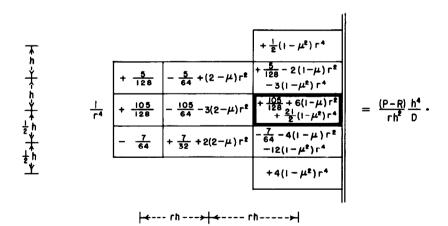
(a) INTERIOR POINT

+n->+-n->+-12++-12->+



| ←- rh-->| ←-- rh ---->|

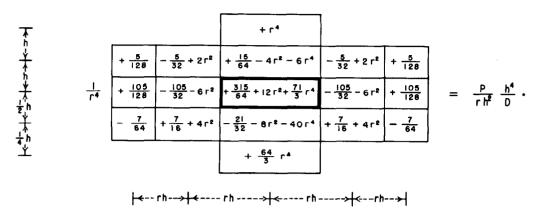
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE



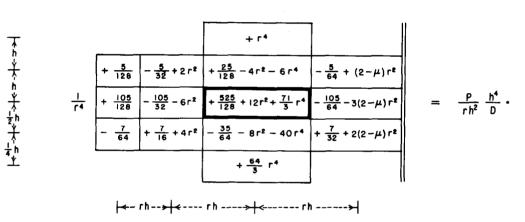
(c) POINT ON A MOMENT-FREE Y-EDGE

Note.—For general notes see Figure 39.

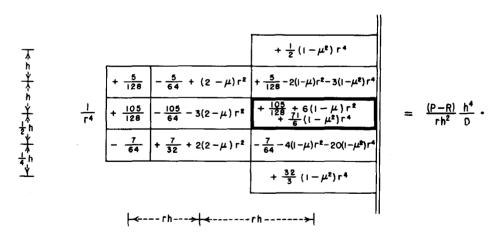
Figure 44.—Load-deflection relations, vertical spacing: 2 at h; 2 at h/2; Sheet VI.



(a) INTERIOR POINT



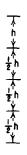
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

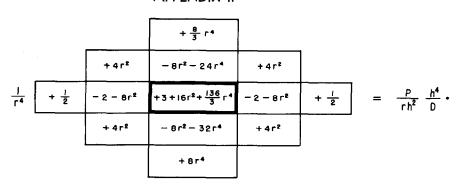


(c) POINT ON A MOMENT-FREE Y-EDGE

Note.—For general notes see Figure 39.

FIGURE 45.—Load-deflection relations, vertical spacing: 2 at h; 1 at h/2; 1 at h/4, Sheet VII.

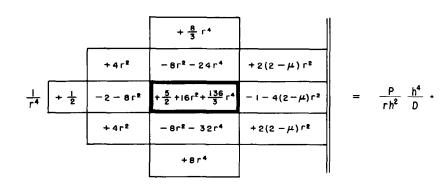




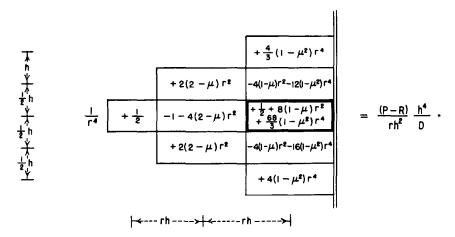
| ←-- rh--->| ←---rh--->| ←---rh--->|

(a) INTERIOR POINT





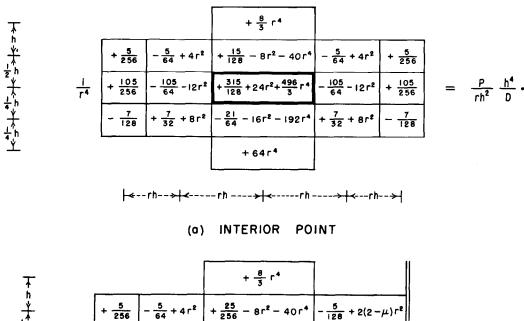
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

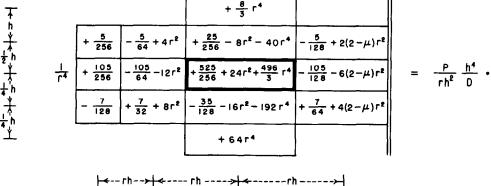


(c) POINT ON A MOMENT-FREE Y-EDGE

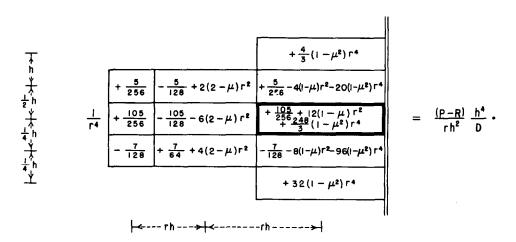
Note.—For general notes see Figure 39.

FIGURE 46.—Load-deflection relations, vertical spacing: 1 at h; 3 at h/2, Sheet VIII.





(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

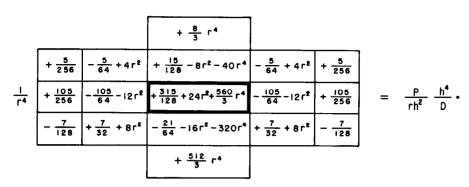


(c) POINT ON A MOMENT-FREE Y-EDGE

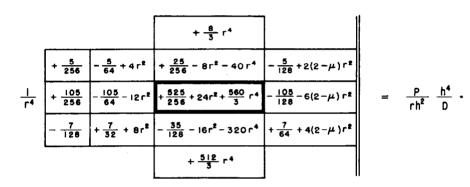
Note.—For general notes see Figure 39.

FIGURE 47.—Load-deflection relations, vertical spacing: 1 at h; 1 at h/2; 2 at h/4, Sheet IX.

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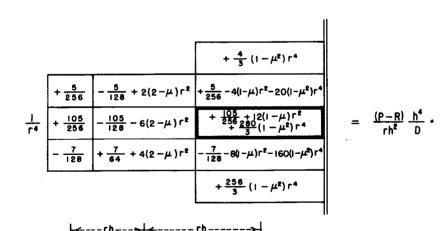






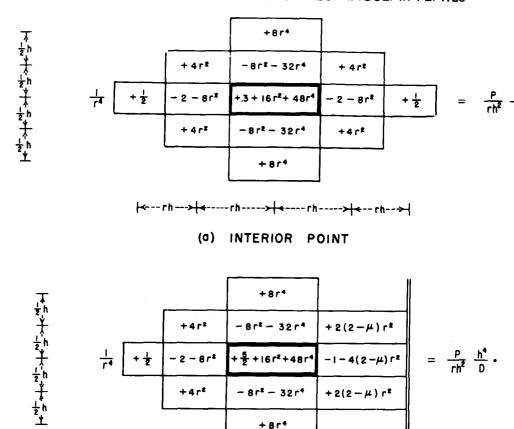
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE





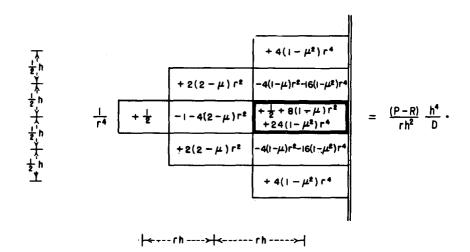
(c) POINT ON A MOMENT-FREE Y-EDGE

Figure 48.—Load-deflection relations, vertical spacing: 1 each at h, h/2, h/4, and h/8, Sheet X.



(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

|---rh--->----rh----->-

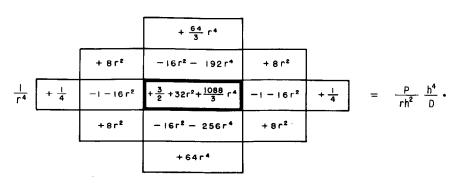


(c) POINT ON A MOMENT-FREE Y-EDGE

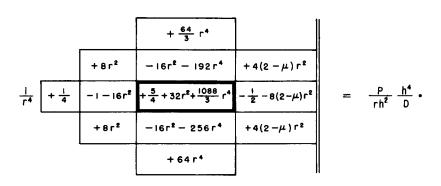
Note.—For general notes see Figure 39.

FIGURE 49.—Load-deflection relations, vertical spacing: 4 at h/2, Sheet XI.

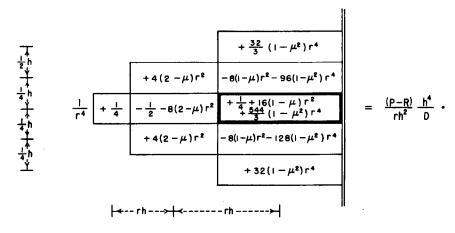






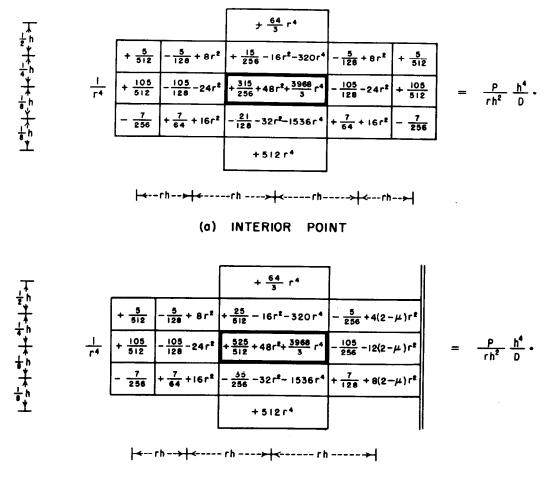


(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

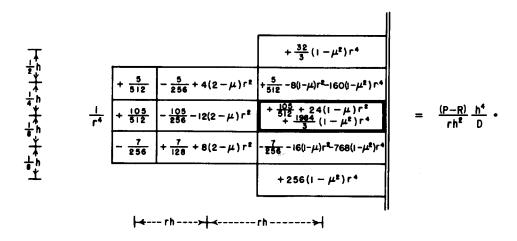


(c) POINT ON A MOMENT-FREE Y-EDGE

FIGURE 50.—Load-deflection relations, vertical spacing: 1 at h/2; 3 at h/4, Sheet XII.



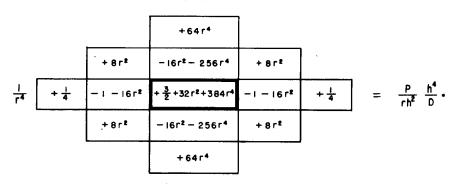
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

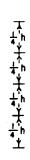


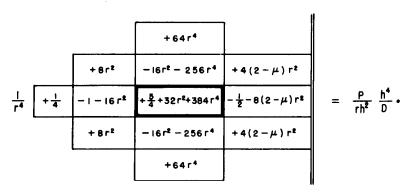
(c) POINT ON A MOMENT-FREE Y-EDGE

FIGURE 51.—Load-deflection relations, vertical spacing: 1 at h/2; 1 at h/4; 2 at h/8, Sheet XIII.



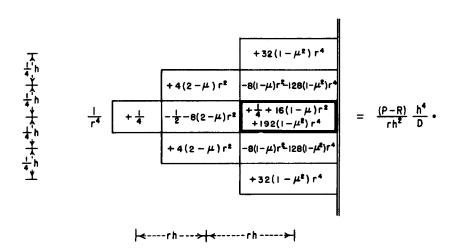






├<--rh--->-|<----rh----->-|

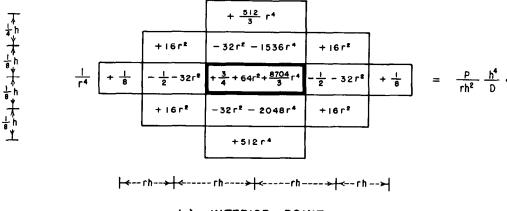
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

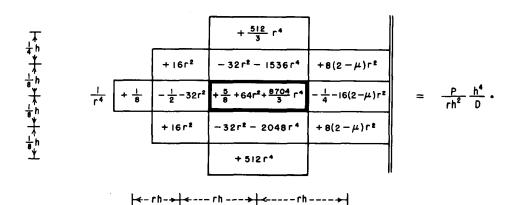


(c) POINT ON A MOMENT-FREE Y-EDGE

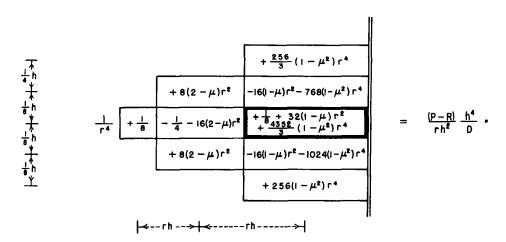
Note.—For general notes see Figure 39.

FIGURE 52.—Load-deflection relations, vertical spacing: 4 at h/4, Sheet XIV.





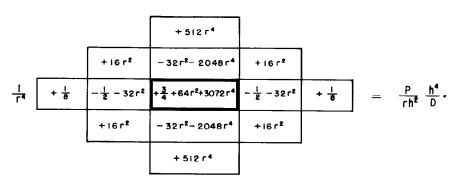
(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE



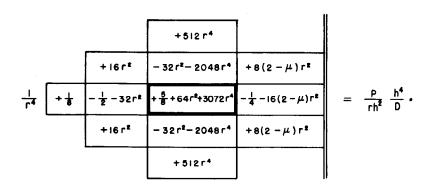
(c) POINT ON A MOMENT-FREE Y-EDGE

Figure 53.—Load-deflection relations, vertical spacing: 1 at h/4; 3 at h/8, Sheet XV.

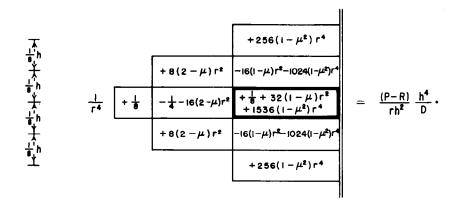








(b) POINT ADJACENT TO A MOMENT-FREE Y-EDGE

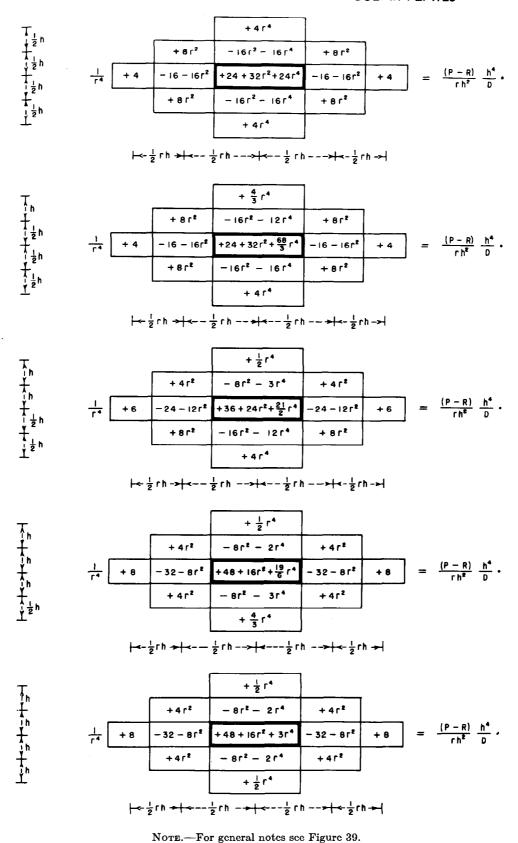


----rh--->-

(c) POINT ON A MOMENT-FREE Y-EDGE

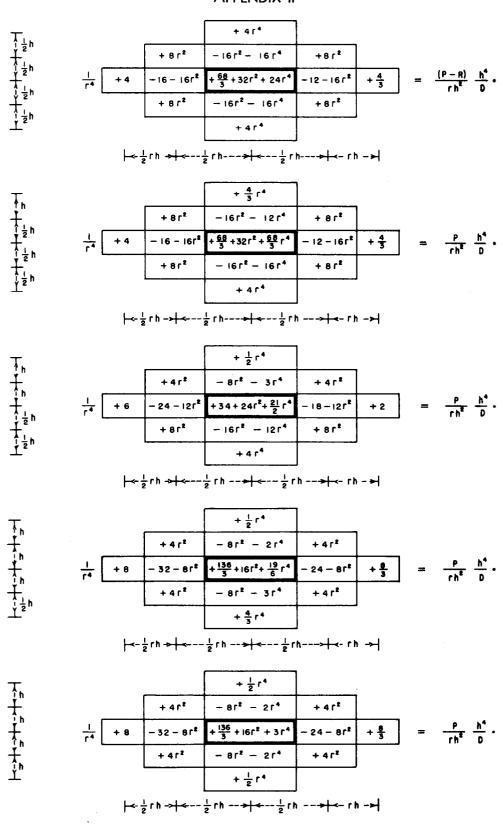
Note.—For general notes see Figure 39.

FIGURE 54.—Load-deflection relations, vertical spacing: 4 at h/8, Sheet XVI.



g g

FIGURE 55.—Load-deflection relations, horizontal spacing: 4 at rh/2, Sheet XVII.



Note.—For general notes see Figure 39.

Figure 56.—Load-deflection relations, horizontal spacing: 3 at rh/2; 1 at rh, Sheet XVIII.

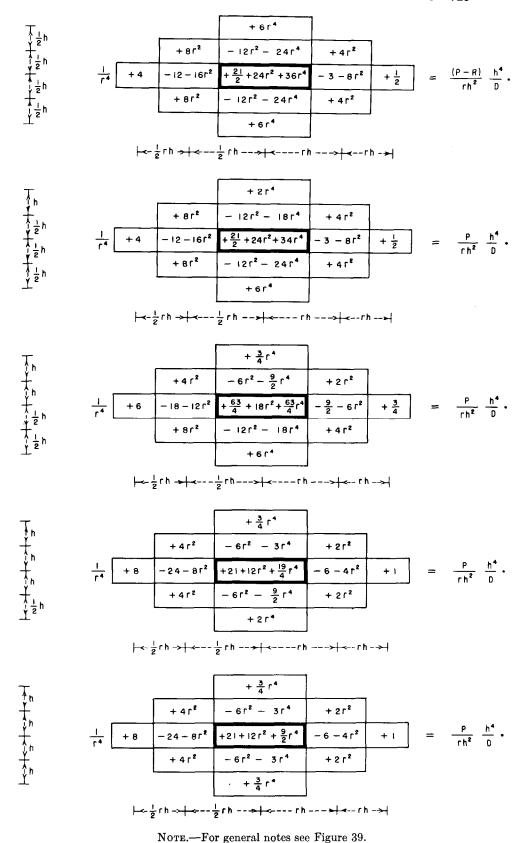


FIGURE 57.—Load-deflection relations, horizontal spacing: 2 at rh/2; 2 at rh, Sheet XIX.

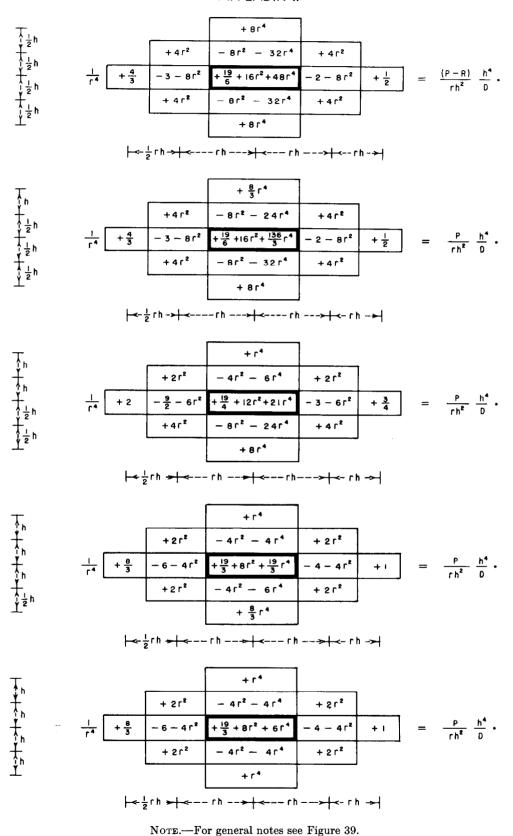


FIGURE 58.—Load-deflection relations, horizontal spacing: 1 at rh/2; 3 at rh, Sheet XX.

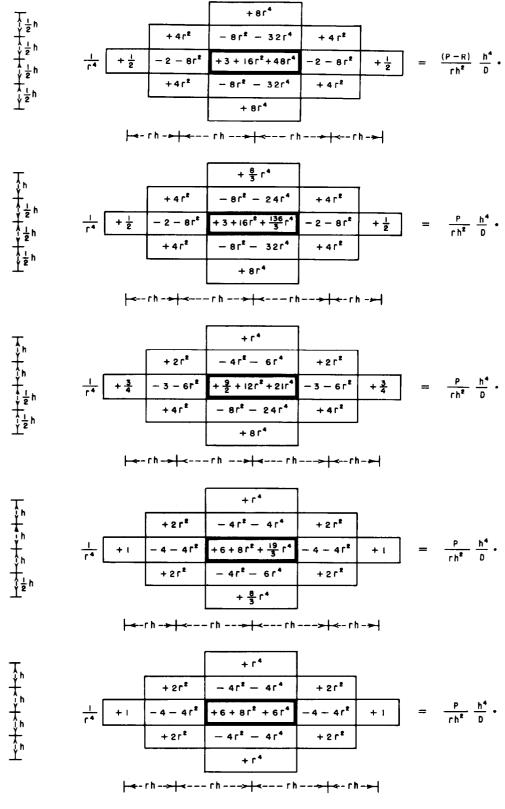
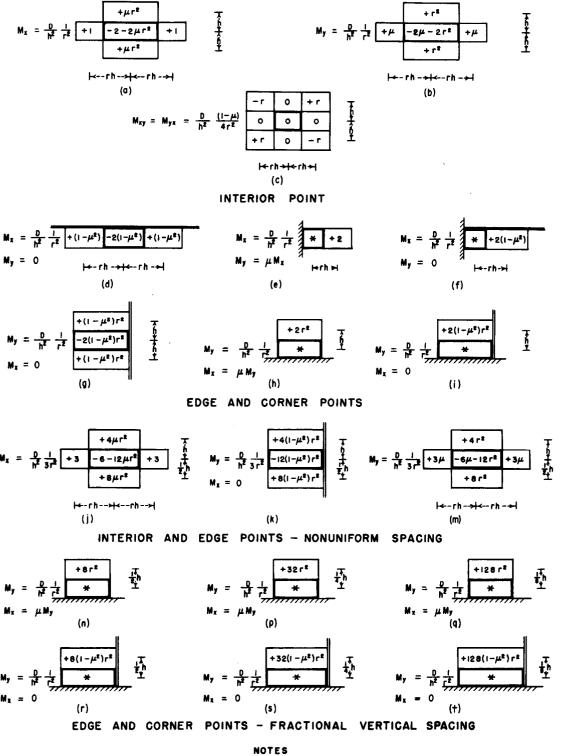


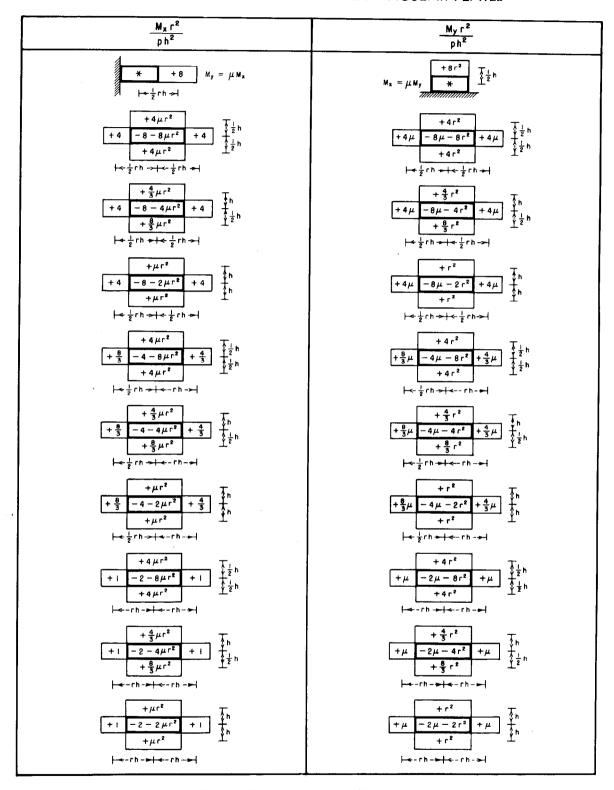
FIGURE 59.—Load deflection relations, horizontal spacing: 4 at rh, Sheet XXI.



either a fixed or moment-free corner. O at any point on a fixed edge.

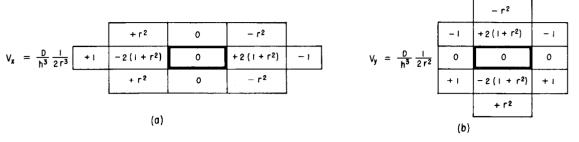
Note.—For general notes see Figure 39.

FIGURE 60.—Moment-deflection relations.

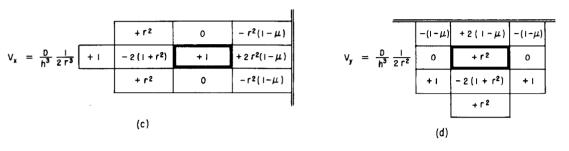


Note.—For general notes see Figure 39.

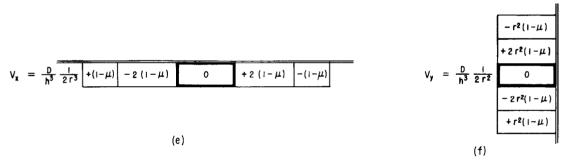
 ${\bf Figure} \ \ 61. -- \textit{Moment-deflection relations, various point spacings}.$



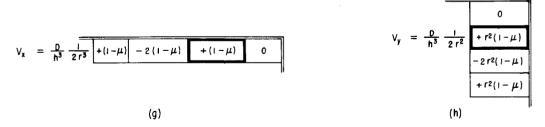
INTERIOR POINT



POINT ADJACENT TO A MOMENT-FREE EDGE



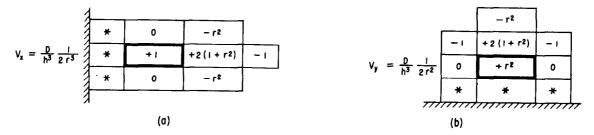
POINT ON A MOMENT-FREE EDGE



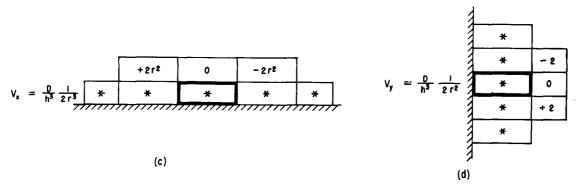
POINT ON A MOMENT-FREE EDGE ADJACENT TO A MOMENT-FREE CORNER

Note.—For general notes see Figure 39.

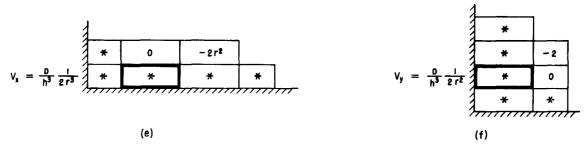
FIGURE 62.—Shear-deflection relations, Sheet I.



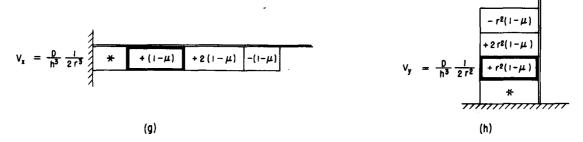
POINT ADJACENT TO A FIXED EDGE



POINT ON A FIXED EDGE



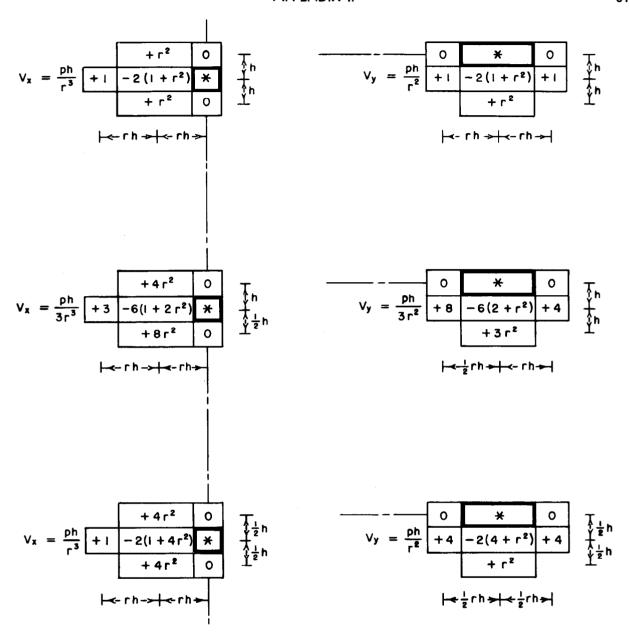
POINT ON A FIXED EDGE ADJACENT TO A FIXED CORNER



POINT ON A MOMENT-FREE EDGE ADJACENT TO A FIXED EDGE

Note.—For general notes see Figure 39.

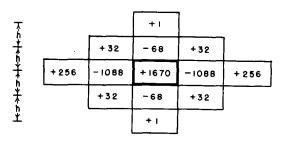
FIGURE 63.—Shear-deflection relations, Sheet II.

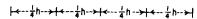


Note: These arrays apply only where the load at corresponding points on opposite sides of the centerline is equal in magnitude but opposite in direction.

 ${\tt Note.}{-\!\!\!\!\!-}{\tt For}$ general notes see Figure 39.

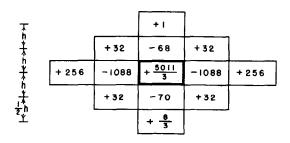
FIGURE 64.—Shear-deflection relations, Sheet III.





Ţ		+28.8	- 59.6	+28.8	
\frac{7}{2}	+256	-1088	+1669	-1088	+256
*		+32	-68	+32	
Ÿ			+1		•

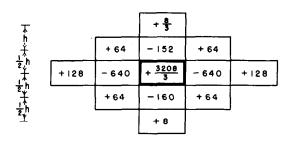
(b) POINT ADJACENT TO A FREE X-EDGE



(C) INTERIOR POINT VERTICAL SPACING: 3 AT h; 1 AT $\frac{1}{2}$ h

- -	+122.88	-517.12	+789.48	-517.12	+122 88	=
Î		 	- 59.6	-28.8	1122,00	
Ť.			+1		J	

(d) POINT ON A FREE X-EDGE



Ţ.			+1		
Ţ	+10	- 8	-10	- 8	+10
<u> </u>	+ 210	-936	+ 4427	-936	+210
±h X	-28	+176	-336	+176	- 28
4 <u>n</u>			+ 64/3		

(e) INTERIOR POINT VERTICAL SPACING: I AT h; 3 AT $\frac{1}{2}$ h

(f) INTERIOR POINT VERTICAL SPACING: 2 AT h; I AT $\frac{1}{2}$ h; I AT $\frac{1}{4}$ h

Figure 65.—Load-deflection coefficients, $r = \frac{1}{4}$, $\mu = 0.2$.

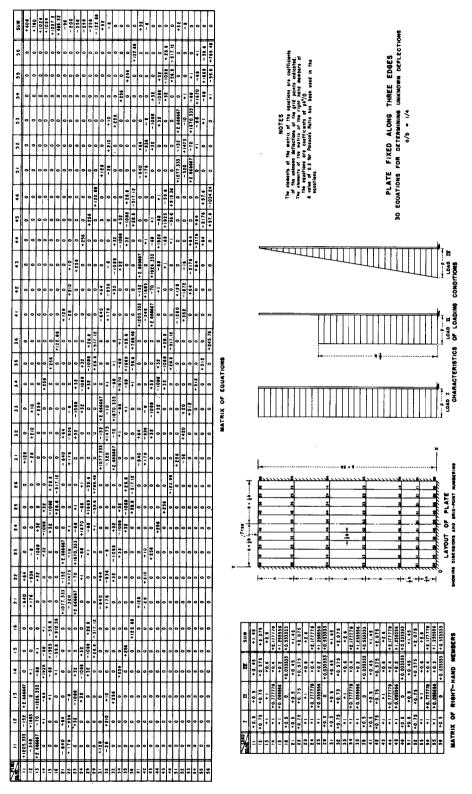
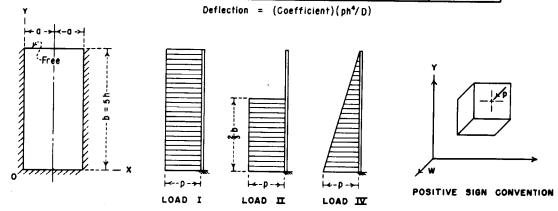


FIGURE 66.—Plate fixed along three edges—30 equations for determining unknown deflections. a/b=1/4.

		DEFL	ECTION COE	FFICIENTS -	30 EQUATI	ons
	y/b ×/a	0.2	0.4	0.6	0.8	1.0
	1.0	+ .017022	+.049680	+.083466	+.107935	+.116792
H	0.8	+.016122	+.046840	+.078499	+.101377	+.109650
₽	0.6	+.016030	+.046526	+.077914	+.100572	+.108761
60	0.4	+ .015353 *	+.044177*	+.073597*	+.094719*	+.102331 *
	0.2	+.011196 +	+.031304 +	+.051265 *	+.065339*	+.070366 *
	0.1	+ . 005859 #	+ 015907 *	+.025588 *	+.032283*	+.034651 *
	1.0	+.000426	+.001800	+.003572	+.005018	+.005570
п	0.8	+.003026	+.009459	+.016489	+.021746	+.023678
٥	0.6	+.011081	+.031691	+.052629	+.067621	+.073018
LOA	0.4	+ .014484 *	+.041246 *	+.068290*	+.087579*	+.094508 *
۲ ,	0.2	+.011123 +	+.030992 *	+.050636 *	+.064448 *	+.069374 *
	0.1	+.005866 *	+ .015887 #	+.025515 *	+.032518*	+ .034506 *
	1.0	+.001780	+.005582	+.009748	+.012870	+.014019
Ħ	0.8	+ .003614	+.010653	+.018006	+.023367	+.025313
	0.6	+.006462	+.018748	+.031388	+.040509	+.043804
LOAD	0.4	+ .008999 *	+ .025735 *	+.042710 *	+.054845*	+.059210*
ا د	0.2	+ .008349 *	+ .023051 +	+.037455 *	+.047522#	+.051102#
	0.1	+ .004804 *	+ .012753 *	+.020243 #	+.025348#	+ .027141 #



		DEFL	ECTION COEF	FICIENTS -	20 EQUAT	IONS
	y/b ×/0	0.2	0.4	0.6	0.8	1.0
I	0.4	+ .015283	+ .043935	+.073156	+ .094124	+.101678
وا	0.2	+ .010730	+.029914	+ .048903	+ .062267	+ . 067035
LOAD	0.1	+ .004899	+ .013281	+ .021325	+ .026868	+ .028824
	0.05	+ .001835	+ .004944	+.007860	+.009833	+ .010522
H	0.4	+ .014414	+.041004	+ .067848	+.086983	+.093855
	0.2	+ .010657	+ .029598	+.048268	+ .061367	+.066034
LOAD	0.1	+ .004900	+ .013246	+ .021229	+.026715	+.028649
	0.05	+ .001840	+ .004945	+ .007847	+ .009805	+.010489
Ħ	0.4	+ .008937	+ .025523	+ .042324	+ .054326	+ .058641
	0.2	+ .007946	+ .021849	+ .035416	+.044873	+ .048232
LOAD	0.1	+ .003980	+ .010505	+ .016603	+ .020734	+.022181
لتا	0.05	+ .001579	+ .004080	+ .006341	+.007838	+ .008356

Deflection = $(Coefficient)(ph^4/D)$

NOTE
Starred values computed from 30 equations are discarded when the corresponding improved value is obtained from the 20 equations.

Figure 67.—Plate fixed along three edges, deflection coefficients. $a/b=\frac{1}{4}$. Various loadings.

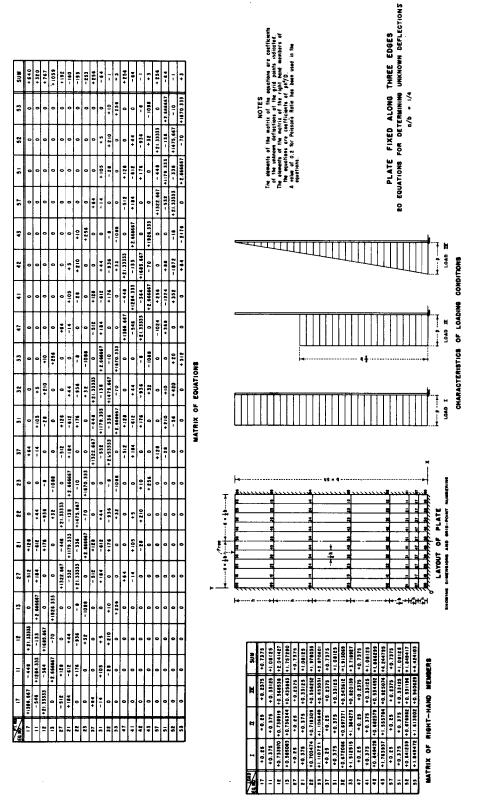


Figure 68.—Plate fixed along three edges—20 equations for determining unknown deflections. $a/b = \frac{1}{4}$.

MOMENTS AND REACTIONS FOR RECTANGULAR PLATES

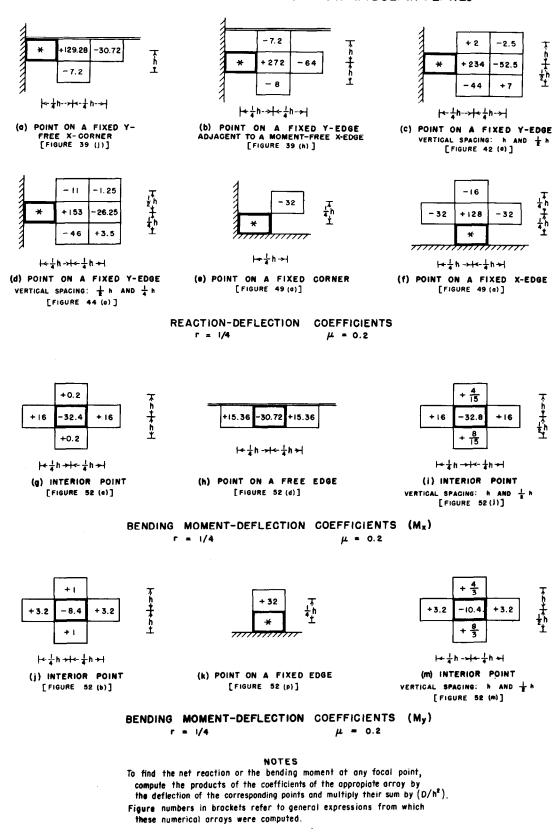


Figure 69.—Numerical values of typical moment and reaction arrays, $r=\frac{1}{4}$, $\mu=0.2$.

POINT NO.	DEFLECTIONS - w/(ph4/D)									
TENS	0	ı	2	3	4	5				
6	0	+.017022	+.049680	+.083466	+.107935	+.116792				
5	0	+.016122	+.046840	+.078499	+.101377	+.109650				
4	0	+.016030	+.046526	+.077914	+.100572	+.108761				
3	0	+.015283	+.043935	+ .073156	+.094124	+.101678				
2	0	+.010730	+.029914	+.048903	+.062267	+.067035				
ı	0	+.004899	+.013281	+ .021325	+.026868	+.028824				
7	0	+.001835	+.004944	+.007860	+.009833	+.010522				
0	0	0	0	0	0	0				

POINT		REAG	TIONS	
POINT NO. 06 05 04 03 02 01 07 00 10 20 30 40 50	P/ph²	DEFL. TERM	R/ph ²	R _t /ph
06	+.0625	+.558356	+.620856	+ .248342
05	+.125	+1.136626	+1.261626	+.252325
04	+.125	+1.131256	+1.256256	+.251251
03	+.125	+1.131056	+1.256056	+,251211
02	+.09375	+.738474	+ .832224	+.190484
01	+.046875	+.178392	+ .225267	
07	+.03125	000992	+.030258	
00	+.015625	058720	043095	+.029514
10	+.03125	001712	+.029538	+.023630
20	+.03125	+.110096	+.141346	+.113077
30	+.03125	+.192016	+ .223266	+.178613
40	+ .03125	+.240512	+ .271762	+.217410
50	+.03125	+.256320	+ .287570	+.230056
		Σ*	+6.249145	* Includes

POINT NO.	BENDING MOMENT - Mx/pb2								
TENS	0	ı	2	3	4	5			
6	+.020917	+.009607	+.000693	005724	009592	010883			
5	+.020636	+.009348	+.000622	005585	009301	010539			
4	+ .020518	+ .009253	+ .000553	005621	009305	- ,010531			
3	+.019562	+.008526	+ .000273	005438	008788	009890			
2	+.013734	+ .005335	000330	003930	005917	006549			
0	0	+.000470	+.001266	+.002012	+.002517	+ .002694			

POINT NO.	BENDING MOMENT - My/pb2								
TENS	0	ı	2	3	4	5			
6	0	0	0	0	0	0			
5	+.004127	+.001901	+.000221	000949	001639	001868			
4	+.004104	+.001825	+ .000023	001284	002078	002344			
3	+.003912	+.001559	000384	001836	002734	003036			
2	+.002747	+.000703	001051	002368	003177	003449			
0	0	+ .002349	+.006328	+.010061	+.012586	+.013468			

Figure 70.—Plate fixed along three edges, deflections—reactions—bending moments, Load I. $a/b = \frac{1}{4}$, $\mu = 0.2$.

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Mission of the Bureau of Reclamation

The Bureau of Reclamation of the U.S. Department of the Interior is responsible for the development and conservation of the Nation's water resources in the Western United States.

The Bureau's original purpose "to provide for the reclamation of arid and semiarid lands in the West" today covers a wide range of interrelated functions. These include providing municipal and industrial water supplies; hydroelectric power generation; irrigation water for agriculture; water quality improvement; flood control; river navigation; river regulation and control; fish and wildlife enhancement; outdoor recreation; and research on water-related design, construction, materials, atmospheric management, and wind and solar power.

Bureau programs most frequently are the result of close cooperation with the U.S. Congress, other Federal agencies, States, local governments, academic institutions, water-user organizations, and other concerned groups.

A free pamphlet is available from the Bureau entitled "Publications for Sale." It describes some of the technical publications currently available, their cost, and how to order them. The pamphlet can be obtained upon request from the Bureau of Reclamation, Attn D-7923A, PO Box 25007, Denver Federal Center, Denver CO 80225-0007.